



# SENSITIVITY ANALYSIS ON FLEXIBLE ROAD PAVEMENT LIFE CYCLE COST MODEL

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## ABSTRACT

*Sensitivity analysis is a tool used in the assessment of a model's performance. This study examined the application of sensitivity analysis on a developed flexible pavement life cycle cost model using varying discount rate. The study area is Effurun, Uvwie Local Government Area of Delta State of Nigeria. In order to determine one of the vital geotechnical engineering properties (i.e. California Bearing Ratio) of the soil samples required for this study, soil samples were taken from the subgrade of the study area using the disturbed sampling method. The laboratory test was carried out at geotechnical laboratory in Warri and in accordance with AASTHO 1993 to determine the subgrade strength. Existing data such as maintenance records, vehicular traffic counts, material costs, discount rate and pavement design data were sourced from the Delta State Ministry of Works, Effurun. These acquired data and CBR results of 5% and 6% were used for the design of three competing flexible pavement systems and Bill of Engineering Measurements obtained. These were used for the evaluation of the Life-Cycle Costs (LCCs) using present worth cost (PWC) method; varying discount rates in MS Excel spread sheets for a design life of 20 years. Regression modeling of the three scenarios was done with sensitivity analysis carried out on the developed model. The project age was used as its independent variable, while discount rate is a secondary independent variable varied by  $\pm 4\%$ ,  $\pm 8\%$  and  $\pm 12\%$  of the initial discount rate of 5.4% applied with the accuracy of 95%. The coefficient of determination ( $R^2$ ) for the Hot Rolled Asphalt (HRA), Interlocking Concrete Pavement Block (ICPB) and Do-nothing models are 0.97, 0.95 and 0.99 respectively indicating a robustness of the developed models. The graphs produced from the sensitivity analysis indicate a decrease in life cycle cost with increasing interest rate for the alternatives. These results are vital for the economic evaluation of flexible pavement and transportation systems.*

**Keywords:** Sensitivity analysis, flexible road pavement, life cycle cost model and Present worth Cost.

## 1. INTRODUCTION

The demand for transport infrastructure systems is enormous due to the ever-growing population, industrialization and urbanization growth in developing countries. The cost of meeting this huge infrastructural demand places so much pressure on government's limited resources [1] and [2].

During the life cycle of an infrastructure so many uncertainties and predictions are involved. These include, design assumptions, construction methodology choice, maintenance strategy and soil testing and results analysis. Road infrastructure like in every other sector requires huge funding and

maintenance. A systematic monetary valuing process is required to justify the amount needed for its planning, design, procurement and pavement maintenance in road infrastructure [3].

The highway engineer can increase confidence in the decision made in transportation infrastructure delivery, in the midst of so much risk in the project delivery, is by carrying out a sensitivity analysis on the life cycle cost model used as the decision support tool [4]; [5]; [6] and [7]. The two most commonly used methods of assessing and managing the risk and uncertainties are probabilistic analysis and sensitivity analysis. The probabilistic approach combines

probability descriptions of analysis inputs to generate the entire range of outcomes as well as the likelihood of occurrence. Probabilistic analysis represents uncertainties more realistically than does a sensitivity analysis. Sensitivity analysis assigns the same weighting to all extreme or mean values, whereas probabilistic analysis assigns the lowest probability to extreme values [8]. In this paper, the sensitivity analysis is examined with a view to cross checking the simple deterministic life cycle cost model earlier developed by the authors.

Sensitivity analysis is a technique used to determine the influence of major data variable resulting from input assumptions, projections and estimates on life cycle cost analysis results [9] and [10]. Major input values are varied while all other input values remain constant and the amount of change in results is noted [11] and [1]. Sensitivity analysis is a necessary tool which helps in figuring out which of the design decision variables in a model is mostly influential or has more relative significance on a decision made. The use of sensitivity analysis for the investigation of the life cycle cost model in pavement infrastructure is relevant following the likely errors and inconsistencies in estimation or prediction costs and financial data, traffic loading prediction, pavement and geometrical design variables.

Generally, when a project is evaluated deterministically over its lifecycle, the planning, design, delivery, maintenance, asset management strategies are generated using expert opinions and experiences based on practice and project history within the planning agency. However, in the life cycle cost studies in flexible pavement delivery, it is important that planned rehabilitation and timing strategy, in-situ material and subsoil tests and traffic loading assessments necessary for the geometrical and pavement design of the traffic corridor be considered. [12] posited that in real life most infrastructure projects can undergo a number of rehabilitation activities at any time during the analysis period and that some of the established design processes entail certain assumptions.

The objectives of the study are to carry out the sensitivity analysis on the life cycle analysis model developed for the road pavement in the study area as well as determine the effect of the sensitivity analysis of the three competing pavement alternatives.

## 2. LITERATURE REVIEW

The initial framework for the life cycle cost model was developed in the late 90s. Transportation infrastructure life cycle cost model are based on statistically derived cost estimating relationships (CER) and various methodology used to predict the cost of such schemes or infrastructures [10]. There are known to consist of specific previous construction, fiscal and cost database, material attributes and maintenance estimates and prediction. These are either estimated from an engineering planning and design exercises or simply predicted to approximate the real world. However the experience and professional judgment of the developer of the model is paramount in the confidence and quality level of the resulting LCC model.

Life cycle cost can be optimized for the great gains of best transportation infrastructure investments in developing countries. Transportation models already exist in the form of the four basic steps of transportation viz: trip generation, distribution, assignment and modal choice or tour based or activity based time. These models include the Delta, PECAS, UPLAN, DEPHI and Urban Sim. From literature, these models are not holistically available for the highway engineer or transportation economics in the developing countries because these models are organizations and specific projects needs based.

Life-cycle cost is suggested as a parameter when selecting road designs or evaluating bids [13] and [14]. Unfortunately, life-cycle cost analyses are still of less importance in bid evaluations due to difficulties related to the absence of reliable data and methods for calculating life-cycle costs for road projects [6].

Lack of maintenance culture and investment related data is attributable to the fact that most road authorities do not have proper methods for systematic data collection and banking regarding the planning, design, construction and maintenance of infrastructure. Absence of reliable life-cycle cost methods is also due to the lack of accurate road deterioration models as well as models for calculating user's or societal costs. Current deterioration models are based on experience and empirical models [4]; such models can give acceptable results, if only the historical circumstances are similar to future circumstances. However, such circumstances seldom exists for road construction due to, among other things, traffic development, use of heavier vehicles and new types of tires and transport systems upgrade and development. Sensitivity and risk analysis are

useful in understanding and handling these envisaged uncertainties to ensure a high confidence level of the results obtainable from the proposed LCC model.

In another Life-Cycle Cost study on the estimation of the environmental impacts of fuel fired cogeneration plants reported in the proceedings of 2000 International Joint Power Generation Conference, [15] opined that any of the variables can be estimated using the Life-Cycle Cost concept for decision making. It is in the process of the sensitivity analysis that the most critical cost variables in the infrastructure life cycle in the life cycle can be detected and effectively managed.

Previous related studies on sensitivity analysis on project evaluation models include the works of [16-24] in a developing country. In the work of [17] input parameters or assumptions used in the model formulation were ranked according to their influence, the baseline valuation criteria or model using the Tornado diagram. This approach is probabilistic and cumbersome leading to more presumptions and thus more risk in the decision making process. In related work, [16] focused on differential importance measure (DIM) mathematical frame work. [25] in their work provided a detailed zone users costs and introduced a probabilistic approach to account for uncertainties.

However, most of these works already done in transportation infrastructure delivery are either complex studies or specific-need based for environments in developed countries design for very heavy annual daily traffic (ADT). In developing countries where the unavailability of project cost evaluation cost models, absence of users cost and road maintenance data bank, poor pavement design and maintenance infrastructure and resources and technological information gaps exists. Our studies is a deterministic one where a case study area typical of the Niger Delta region is examined in an urban environment with a low design speed and traffic annual daily volume(ADT).

This work tests the flexible pavement cost model with varying interest rate for an already procured, developed and completed urban road infrastructure corridor using Microsoft Excel and SPSS v20 software [24]. It examines how the variation in interest rates affects the present worth life cycle cost for the competing pavement alternatives in Million Naira/Kilometer.

### 3. MODEL FORMULATION

The present worth method of economic evaluation involves the conversion of all the present and future estimated expenses to a base of today's costs using an appropriate interest rate. The total of the computed present value costs are then compared with one another. The present worth of costs method is directly comparable to the equivalent uniform annual cost method for comparable benefits [26]. In this study, a limitation to the present worth cost method is made for clarity using tropical information from field practice, experience and agency costs. The general expression for the Present Worth Cost of a transportation infrastructural project is given by [27].

$$PWC = Initial \cos t + \sum_{k=1}^N Rehab \cos t \frac{1}{[(1+i)^{nk}]}$$
 (1)

In (1), *i* is the interest rate, *n* is the year of expenditure or age of infrastructure,  $\frac{1}{[(1+i)^{nk}]}$  = present value factor. Following equation (1) above, a linear relation for the life cycle cost (LCC) for the competing pavement alternatives are represented in a linear relationship as shown in the model form below.

$$Y = b + mx + e$$
 (2)

Here, *Y* is the Dependent variable, deterministic Present worth Cost of alternative Pavement or infrastructure or scheme in Million Naira, *b* is the Independent variable, regression constant or vertical intercept on the cost axis Agency's (Pre-construction+ Initial Construction) in Million Naira, *m* is the Regression coefficient or slope of trend line relating the cost and the entire project life (a function of the annual expenditure, salvage value, analysis period, discount rate), *x* is the Independent variable, particular period or year of interest of projection of the present worth cost in year, and *e* is the Independent variable, other costs (such as socio-economic and political cost element and error term due to uncertainty in data analyses.

### 4. METHODOLOGY

The soil samples were taken from the subgrade of the study area using the disturbed sampling method and were transported to the Geotechnical engineering laboratory in Warri for California Bearing Ratio (CBR) laboratory tests on the samples. The CBR test, which is a vital geotechnical engineering properties required for the design of flexible pavement, was carried out in accordance with the American Association of State highway and Transportation Officials (AASHTO) in

order to determine the in-situ subgrade strength as required for flexible pavement design [28]. The study was carried out using existing input data such as: maintenance records, vehicular traffic counts, material and market survey reports, discount rate, the reports of route investigation, geometric and pavement design and cost data sourced from the Delta State Ministry of Works, Effurun, Nigeria. The acquired existing data and the soil subgrade CBR test result of 4% were used for the design of three competing flexible pavement systems; Hot Rolled Asphalt (HRA), Interlocking Concrete Block Pavement (ICBP) and a third; Do-nothing scenario pavement surface courses.

The input data and variables obtained from the Ministry of Works, Effurun, Delta State, Nigeria also includes maintenance records, average commercial vehicle counts per day of 400 of a single carriageway from which the future traffic Estimated Standard Axles Loading (ESAL) of  $5.27 \times 10^6$  was calculated. A factor of safety of 1.2 was allowed for overloading and constant exposure of pavement to seasonal over flooding due to the flatness of the terrain for a period of 20years and 4% growth rate. In the location of study, a design speed of 60km/hr, applicable to urban roads was used in the geometric design for single carriage way.

From the alternative sectional full depth designed, the Bill of Engineering Measurements and Evaluation (BEME) was developed for the design alternatives. These were later used for the evaluation of the Life-Cycle Costs (LCCs) using present worth cost (PWC) method and varying discount rates in MS Excel spreadsheets for a design life of 20 years. For the third

alternative, no surfacing course was provided (Do-Nothing). This implies that, sub-base and base course are common to all the three alternatives.

Regression modeling with computer software’s (SPSS and MS Excel) package for all three scenarios was done. The sensitivity analysis was carried out on the developed model. The project age was used as its independent variable, while discount rate is a secondary independent variable varied by  $\pm 4\%$ ,  $\pm 8\%$  and  $\pm 12\%$  of the initial discount rate of 5.4% applied. This was done on the MS Excel spreadsheets and SPSS Version 20 with the accuracy of 95%. The variations in the outcomes of the life cycle cost with the varying interest are presented in the sensitivity analysis graphs.

**5. RESULTS AND DISCUSSION**

**5.1 Results**

Fig. 1 presents the typical full depth cross sections alternatives of the urban road. Table 1 is the summary of Bill of Engineering Measurements and Evaluation of the competing pavement alternatives.

Tables 2-4 show the summary of the statistical output for the three (3) competing pavement alternatives models. The combined life-cycle costs (LLCs) and the pavement age plot for the three competing flexible pavement alternatives at 6.2% interest rate is shown in Fig. 2.

Figs. 3-8 present the Life-Cycle Costs versus Pavement age and Normal probability percentile plots for each of the competing pavement alternatives. Fig. 9 presents the Life-Cycle Costs versus Interest rate sensitivity graph for the models developed.

*Table 1: Summary of Bill of Engineering Measurements and Evaluation for the competing alternatives (Adapted from Proposed Delta State Ministry of Works BEME, 2011)*

Item No.	Description	Unit	Qty	Rate (N)	Amount (N)
<b>BILL NO. 4: PAVEMENT AND SURFACING (HRA)</b>					
4.01	Provide and lay prime coat of MC0/MC1 cut back bitumen at a rate of 0.9litres/sq.m including sharp sand or quarry dust blinding	m <sup>2</sup>	11,400	250	2,850,000.00
4.02	Provide and apply butiminous emulsion tack coat on the entire carriage way	m <sup>2</sup>	11,400	220	2,508,000.00
4.03	Provide and lay 50mm compacted thickness of HRA concrete binder course of 5% bitumen content.	m <sup>2</sup>	11,400	3,000	34,200,000.00
4.04	Provide and apply butiminous emulsion tack coat on the entire carriage way	m <sup>2</sup>	11,400	220	2,508,000.00
4.05	Provide and lay 40mm compacted thickness of HRA wearing course of 6% bitumen content.	m <sup>2</sup>	11,400	2,700	30,780,000.00
	Total Bill No. 4 Carried to Summary				72,846,000.00
<b>BILL NO. 4: PAVEMENT &amp; SURFACING (ICPB)</b>					
4.01	Provide and lay damp proof and appropriate filter membrane	m <sup>2</sup>	11,400	200	2,280,000.00

Item No.	Description	Unit	Qty	Rate (N)	Amount (N)
4.02	Provide and lay 75mm thick approved interlocking stones compacted sharp bedding on the entire carriage way including kerbs	m <sup>2</sup>	11,400	4,000	45,600,000.00
	Total Bill No. 4 Carried to Summary				47,880,000.00
<b>BILL NO. 4: PAVEMENT AND SURFACING (DO-NOTHING)</b>					
4.01	Provide and lay prime coat of MC0/MC1 cut back bitumen at a rate of 0.9litres/sq.m including sharp sand or quarry dust blinding	m <sup>2</sup>	11,400		-
4.02	Provide and apply butiminous emulsion tack coat on the entire carriage way	m <sup>2</sup>	11,400		-
4.03	Provide and lay 50mm compacted thickness of HRA concrete binder course of 5% bitumen content.	m <sup>2</sup>	11,400		-
4.04	Provide and apply butiminous emulsion tack coat on the entire carriage way	m <sup>2</sup>	11,400		-
4.05	Provide and lay 40mm compacted thickness of HRA wearing course of 6% bitumen content.	m <sup>2</sup>	11,400		-
	Total Bill No. 4 Carried to Summary				Nil cost for Surfacing

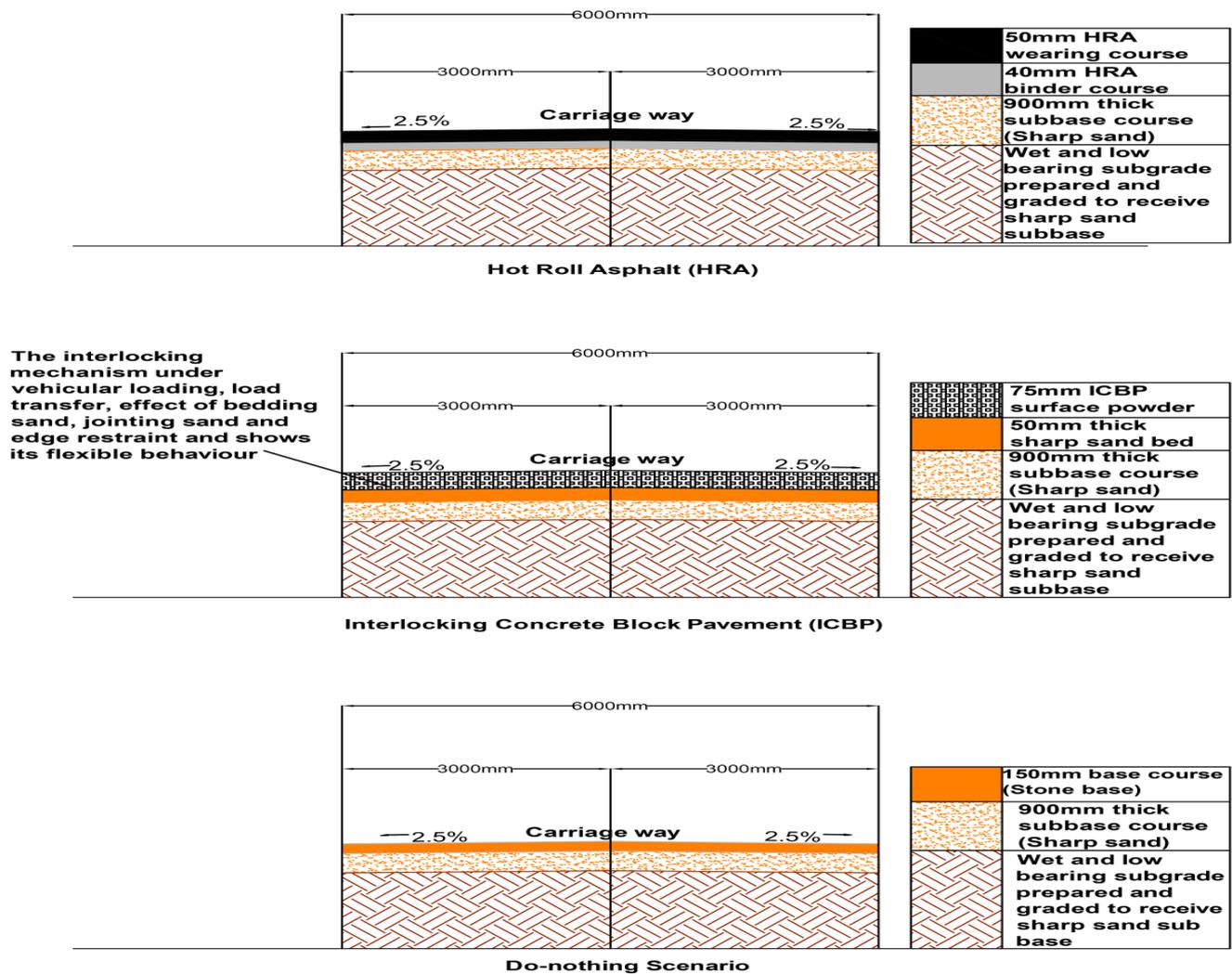


Fig. 1: Typical cross section of the road used for this study

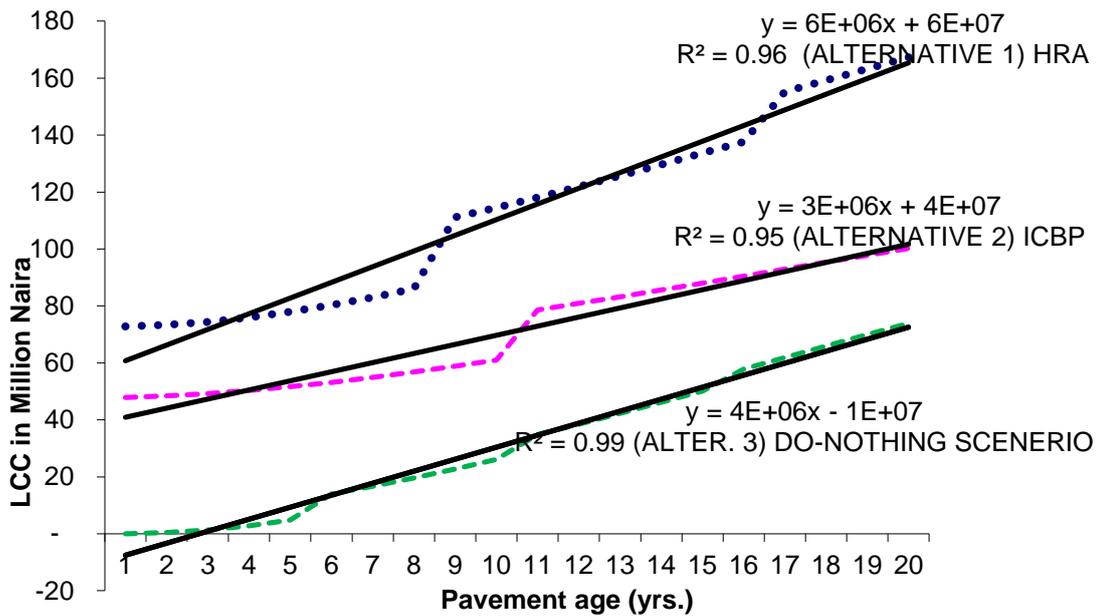


Fig. 2: Combined LCC versus Pavement age plots for the three competing flexible pavement alternatives at 6.2% interest rate. (The bold lines represent the cost model for the alternatives considered).

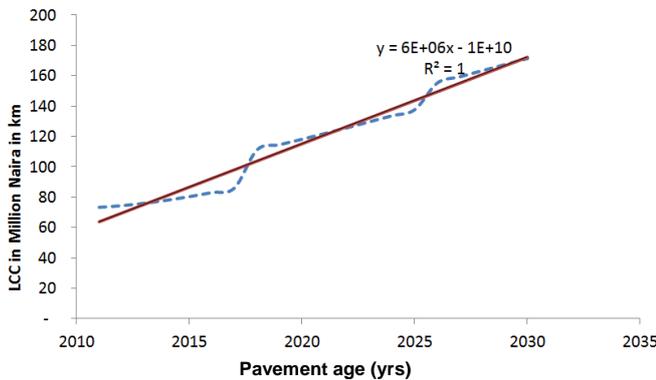


Fig. 3: Life Cycle Cost vs Pavement age plot for HRA model

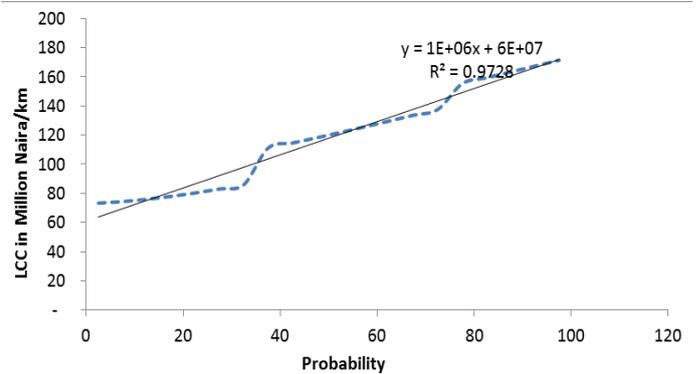


Fig. 4: Life Cycle Cost vs Probability plot for HRA model

Table 2: Summary of the output of Life-Cycle Cost for HRA model

			Regression Statistics				
Multiple R			0.99				
R Square			0.97				
Adjusted R square (R <sup>2</sup> )			0.97				
Standard Error			5800977.89				
Observations			20.00				
			ANOVA				
	df	SS	MS	F	Significance F		
Regression	1	2.1635E+16	2.2E+16	642.92	1.5E-15		
Residual	18	6.0572E+14	3.4E+13				

Total	19	2.2241E+16						
	Coefficients	Standard Error	t Stat	P-value	Lower 95%	Upper 95%	Lower 95.0%	Upper 95.0%
Intercept	-11406645573	454517920	-25.096	1.9E-15	-1E+10	-1E+10	-1E+10	-1E+10
2010	5703860.325	224952.274	25.3559	1.5E-15	5231253	6176468	5231253	6176468
		Residual output			Probability output			
Observation	Predicted 72846000	Residuals	Percentile	72,846,000				
1	63,817,541.20	9528458.798	2.5	73,346,000				
2	69,521,401.53	4890115.373	7.5	74,411,517				
3	75,225,261.85	749554.4887	12.5	75,974,816				
4	80,929,122.18	-2954976.928	17.5	77,974,145				
5	86,632,982.50	-6279718.191	22.5	80,353,264				
6	92,336,842.83	-9275828.17	27.5	83,061,015				
7	98,040,703.15	-11989786.81	32.5	86,050,916				
8	103,744,563.48	7384454.173	37.5	111,129,018				
9	109,448,423.80	5112241.776	42.5	114,560,666				
10	115,152,284.13	3007230.368	47.5	118,159,514				
11	120,856,144.45	1038201.431	52.5	121,894,346				
12	126,560,004.78	-822988.3203	57.5	125,737,016				
13	132,263,865.10	-2601659.602	62.5	129,662,206				
14	137,967,725.43	-4320544.208	67.5	133,647,181				
15	143,671,585.75	-6000000.997	72.5	137,671,585				
16	149,375,446.08	5844463.783	77.5	155,219,910				
17	155,079,306.40	4191295.079	82.5	159,270,601				
18	160,783,166.73	2528794.336	87.5	163,311,961				
19	166,487,027.05	844245.3087	92.5	167,331,272				
20	172,190,887.38	-873551.6825	97.5	171,317,336				

Table 3: Summary of the output of the Life Cycle Cost for Interlocking Concrete Block

		Regression Statistics						
Multiple R	0.98							
R Square	0.96							
Adjusted R square (R <sup>2</sup> )	0.95							
Standard Error	4298457.57							
Observations	20.00							
		ANOVA						
	df	SS	MS	F	Significance F			
Regression	1	7.14009E+15	7.14E+15	386.4369	1.3E-13			
Residual	18	3.32581E+14	1.85E+13					
Total	19	7.47267E+15						
		Standard Error	t Stat	P-value	Lower 95%	Upper 95%	Lower 95.0%	Upper 95.0%
Intercept	-6546638279	336792526.1	-19.4382	1.57E-13	-7E+09	-5.8E+09	-7.3E+09	-6E+09

2010	3276733.402	166687.0352	19.658	1.3E-13	2926537	3626930	2926537	3626930
RESIDUAL OUTPUT				PROBABILITY OUTPUT				
Observation	Predicted 47880000	Residuals		Percentile	47,880,000			
1	42872593.72	5,507,406.28		2.5	48,380,000			
2	46149327.12	3,048,395.15		7.5	49,197,722			
3	49426060.53	868,304.64		12.5	50,294,365			
4	52702793.93	- 1,068,219.28		17.5	51,634,575			
5	55979527.33	- 2,793,353.29		22.5	53,186,174			
6	59256260.73	- 4,336,347.26		27.5	54,919,913			
7	62532994.14	- 5,723,756.29		32.5	56,809,238			
8	65809727.54	- 6,979,655.66		37.5	58,830,072			
9	69086460.94	- 8,125,839.87		42.5	60,960,621			
10	72363194.34	6,232,010.50		47.5	78,595,205			
11	75639927.75	5,248,088.64		52.5	80,888,016			
12	78916661.15	4,320,408.27		57.5	83,237,069			
13	82193394.55	3,434,588.07		62.5	85,627,983			
14	85470127.95	2,577,734.18		67.5	88,047,862			
15	88746861.36	1,738,315.60		72.5	90,485,177			
16	92023594.76	906,048.94		77.5	92,929,644			
17	95300328.16	71,792.13		82.5	95,372,120			
18	98577061.56	- 772,553.89		87.5	97,804,508			
19	101853795	- 1,634,135.72		92.5	100,219,659			
20	105130528.4	- 2,519,231.12		97.5	102,611,297			

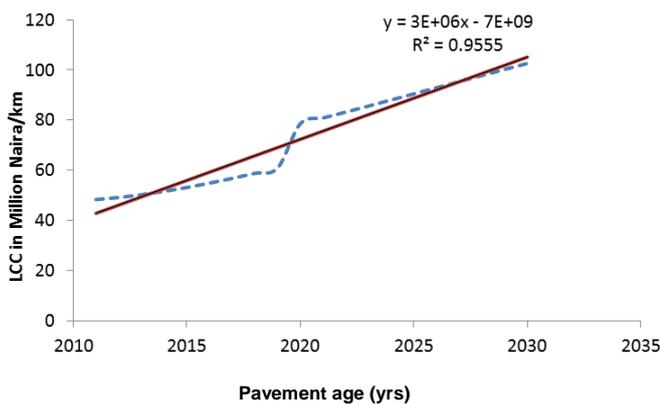


Fig. 5: Life Cycle Cost vs Pavement age plot for ICPB model

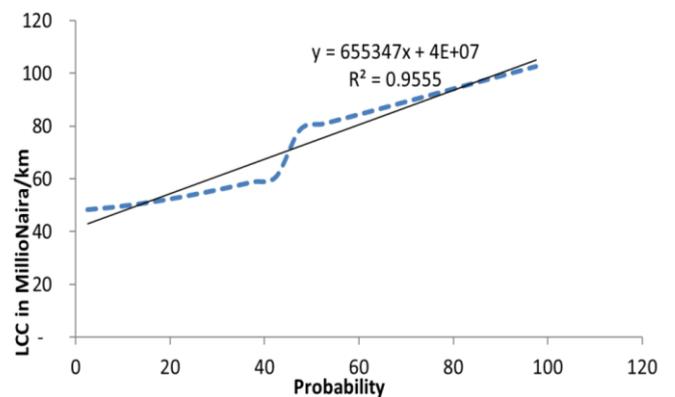


Fig. 6: Cycle Cost vs Probability plot for ICPB model

Table 4: Summary of the output of Life Cycle Cost for Do-Nothing Pavement model

Multiple R	1.00						
R Square	0.99						
Adjusted R square (R <sup>2</sup> )	0.99						
Standard Error	2255531.29						
Observations	20.00						
			ANOVA				

	df	SS	MS	F	Significance F			
Regression	1	1.282E+16	1.3E+16	2519.9	8.4E-21			
Residual	18	9.15736E+13	5.1E+12					
Total	19	1.29115E+16						
	Coefficients	Standard Error	t Stat	P-value	Lower 95%	Upper 95%	Lower 95.0%	Upper 95.0%
Intercept	-8834860757	176725271.2	-49.9921	9E-21	-9.2E+09	-8E+09	-9E+09	-8.5E+09
2010	4390688.178	87465.75179	50.19894	8E-21	4206929	4574447	4E+06	4574447
	Residual output				Probability output			
Observation	Predicted 0	Residuals		Percentile	-			
1	-5186830.279	5,586,830.28		2.5	400,000			
2	-796142.1004	2,167,497.04		7.5	1,371,355			
3	3594546.078	- 748,556.44		12.5	2,845,990			
4	7985234.256	- 3,223,404.17		17.5	4,761,830			
5	12375922.43	1,648,447.57		22.5	14,024,370			
6	16766610.61	- 108,515.09		27.5	16,658,096			
7	21157298.79	- 1,579,004.82		32.5	19,578,294			
8	25547986.97	- 2,805,446.89		37.5	22,742,540			
9	29938675.15	- 3,826,289.33		42.5	26,112,386			
10	34329363.33	477,311.97		47.5	34,806,675			
11	38720051.5	- 233,341.57		52.5	38,486,710			
12	43110739.68	- 832,956.88		57.5	42,277,783			
13	47501427.86	- 1,347,041.41		62.5	46,154,386			
14	51892116.04	- 1,798,502.84		67.5	50,093,613			
15	56282804.22	1,607,103.71		72.5	57,889,908			
16	60673492.39	1,221,498.17		77.5	61,894,991			
17	65064180.57	843,306.51		82.5	65,907,487			
18	69454868.75	458,012.65		87.5	69,912,881			
19	73845556.93	52,770.18		92.5	73,898,327			
20	78236245.11	2,440,281.36		97.5	80,676,526			

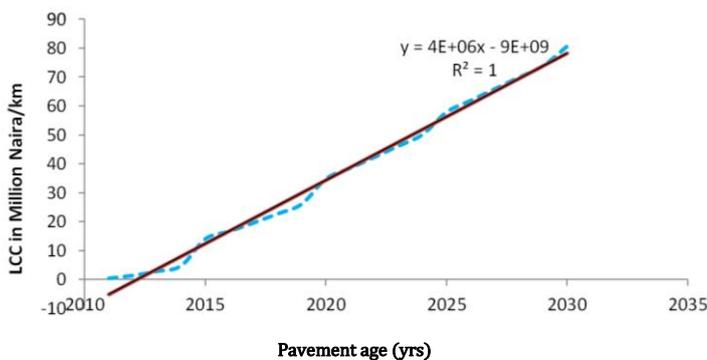


Fig. 7: Life Cycle Cost vs Pavement age plot for Do-Nothing model

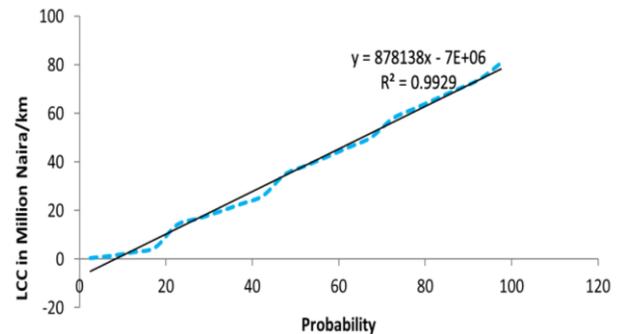


Fig. 8: Life Cycle Cost vs Probability plot for Do-Nothing model

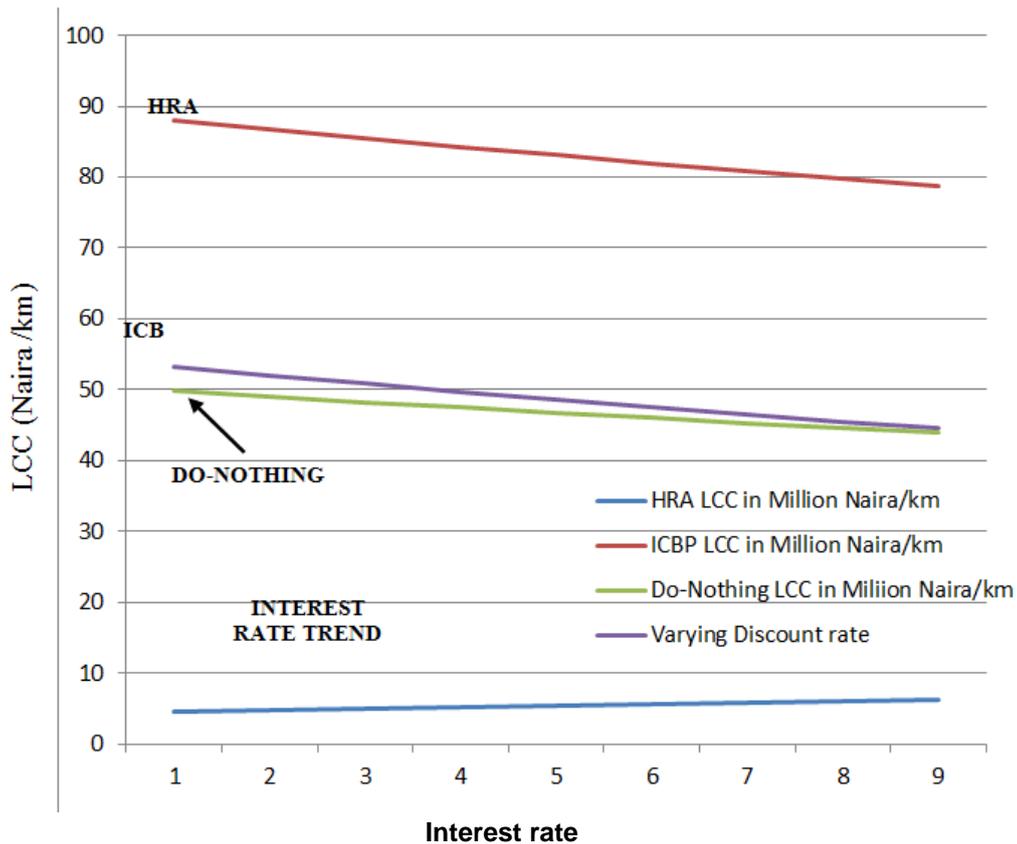


Fig. 9: Life Cycle Cost vs Interest Rate Sensitivity Graph for the three competing alternatives.

6. DISCUSSIONS

All the options considered in the sensitivity analysis in this study have the same baseline conditions and designed performance level of service (LOS). The values of coefficient of determination ( $R^2$ ) for the Hot Rolled Asphalt (HRA), Interlocking Concrete Pavement Block (ICPB) and Do-nothing models (shown in Fig. 1) are 0.97, 0.95 and 0.99 respectively. These values are close to unity (one) which indicates a robustness of the models replicating the data used in the analysis.

The HRA and ICPB alternatives met the required level of services (LOS) while the Do-nothing scenario does not meet the level of service based on the reduced speed, poor driving characteristics, potholing and poor aesthetics observed in the Do-Nothing alternative that is without a binder or wearing surface course after the rainy season. The Do-Nothing alternative is therefore an unacceptable option.

The Sensitivity Analysis of the Life Cycle Cost for the alternatives presented in the interest rate sensitivity graph (Fig. 9) indicates a decrease with increasing interest rates for the three alternatives. The result also shows that the initial agency cost of the procurement of flexible pavement infrastructure and discount rate are significant parameters when an economic

evaluation is done using the present worth cost method. The procedure and concept study are applicable in allied problem areas and sectors. The sensitivity analysis also indicates a decrease in life cycle cost (LCC) in Million Naira/Kilometer with increasing interest rate for the three alternatives.

As indicated in Tables 2-4, the significant value is much less than 0.05 rejection of a null hypothesis that  $B=0$  is adequately based on the assumption that the error term  $e$  (see equation 2) in the linear regression LCCA model is independent of  $X$  and normally represented with zero mean and constant variance. The models Independent variable (project age in years) can significantly predict the dependent variable (LCC) in Millions of Naira per kilometer.

It is observed that the results of the sensitivity analysis have no effect on the R-square values or Goodness of fit statistics. The goodness of fit of the developed economic models as observed from the R-square values which range from 0.95 to 0.99 shows robustness. The SPSS analysis has a Durbin-Watson statistic coefficient of 0.95 (shown in Fig. 2). This show that the LCC model is highly robust in replicating the project data used in the analysis.

Statistical parameters in Figs. 2-8 indicate that the cost models are between 0.950 - 0.999 with

significance value of less than 0.001. Statistically, the parameters obtained in this study are in conformity with the result of [29] in his study on "Cost Model for Pre-and Post-Haulage Road Freight Transport to and from the Intermodal Terminal" in Sweden, which had a mean square value ( $R^2$ ) of 0.96 with a significance value of (F) of 0.001.

## 7. CONCLUSION

The study has shown that sensitivity analysis on the Life- Cycle Cost (LCC) model is necessary to achieve optimality and higher confidence level in model guided decision formulation especially for transport infrastructural delivery with high investment risk, challenges and funds.

Sensitivity analysis is often performed on developed Life-Cycle Cost Analysis (LCCA) models in line with available funds, available technology, construction supervisory experience, competency of contractors, interest rate, effective construction season etc. The sensitivity analysis of the life cycle cost in the study area has shown a decrease with increasing interest rates for the three alternatives.

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