# Water Pipeline Network Analysis Using Simultaneous Loop Flow Correction Method 

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#### Abstract

A water pipeline network analysis with a case study of Owerri in Imo State, Nigeria municipal water reticulated system has been undertaken. What prompted this study is that the case study has a lot of fluctuations in its head loss. Also, the discharge is not proportional to the pipe diameter. The study therefore adopted simultaneous loop flow correction method because it computes simultaneous flows corrections for all loops, hence, the best since computational procedures takes into account the iterative influence of flow corrections between loops which have common pipes. After applying the simultaneous loop flow correction analyzer in a twenty-four sampled pipeline network, a drastic reduction in head loss and regular line along the axis was observed. Besides, the rate at which the water flows was observed to be proportional to the pipe diameter. Hence, the method is a useful aid in planning, designing and operating of reticulated pipeline network for higher efficiency and improved economy.


KEYWORDS: Water Reticulated System, Simultaneous Loop Flow, Iteration, Flow Rate, Pipeline Network, Head Loss

### 1.0 Introduction

Water is an essential natural resource for industrial and natural process, for example, it is used for oil refining, liquid extraction in hydrometallurgical process, cooling, scrubbing in the iron and steel industry and several operations in food processing facilities. Water is an essential input to achieve some desired outcomes, including health and income. Water affects sanitation and hygiene because lack of access to water leads to unhygienic behavior. Water supply is the provision of water by public utilities, commercial organizations, community endeavors or by individuals, usually via a system of pumps and pipes. Water supply systems get water from various locations, including ground water (aquifers) surface water (lakes and rivers) conservation and the sea through desalination. The water is then in most cases purified, disinfected through chlorination and sometimes fluoridated. Treated water then either flows by gravity or pumped reservoirs, which can be elevated such as water towers.

Analysis and design of pipe networks create a relatively complex problem, particularly if the network consists of a range of pipe as frequently occurs in water distribution systems. In the absence of significant fluid acceleration, the behavior of a network can be determined by a sequence of steady state conditions, which form a small but vital component for assessing the adequacy of a network.
In 2010, about $86 \%$ of the global population 96.74 billion people had access to piped water supply through house connections or to an improved water source through other means than house, including standpipes, water kiosks, protected springs and protected wells. However, about 14\% (884 million people) did not have access to an improved water sources and had to use unprotected wells or springs, canals, lakes or rivers for their water needs [13].

Greater number of people having access to pipe water receive poor quality of service, especially in developing countries where about $80 \%$ of the world population lives, the United Nations (2006) revealed that total water availability per capita in Nigeria
decreased from $2514 \mathrm{~m}^{3}$ per year in 2000 to $2250 \mathrm{~m}^{3}$ per year in 2005. In Nigeria, only water as at 2010 as reported in Thisday News paper of August 19, 2010 under the heading, Nigeria, only 17.2 percent have access to potable water. In light of this, this year's world water day centers on water and food security as decline access to water affects agricultural produces. Therefore, there is a dire need for adequate planning, design and provision of reticulated water to every nook and cranny of this nation to ensure a healthy living standard for all, boost agricultural produces and provide enough water for industrial purposes.

## Materials and Method

A pipe network is a set of pipes which are interconnected in such a way that flow from a given input get to a given outlet. An attempt to apply Bernoulli's equation and the continuity of flow equation to the various elements in the network would
lead to a very large number of simultaneous equations which would be cumbersome to solve.

In this work, simultaneous loop flow technique was used to determine the discharge (flow rate) connections in each pipe; head losses in each pipe and Node pressures. Minor losses due to pipe fittings such as valves; pipe bends, elbows, etc can be accounted for by using the equivalent length of pipe method.

The waterworks of Owerri Municipal Area was adopted as a case study. It covers some parts of Owerri Municipal council and Owerri West Local Government Area. The pipe lengths, various pipe diameters, volume flow rates, piping materials, types of joints, and other relevant information was collected from the water board. These values were used in this analytical work. The network diagram of ISWC distribution is shown in Figure 1.


Figure 1 (the network diagram of ISWC)

## Solution of Network Equations

Direct solution of systems of non-linear simultaneous equation is not feasible; hence, it is necessary to use iterative solution methods.

Generally, these methods start with an estimated solution which is interactively refined by repeated corrections until the deviation from the true solution is reduced to an acceptable tolerance value. Hardy Cross $(\mathrm{HC})$ method is the most widely used technique for
solving for the unknown in water network analysis. It is based on a loop iterative computation.

Newton-Raphson method is a better technique for solving the network problems; however, the method adopted here computes simultaneous flow corrections for all loops, hence, the best since the computational procedure takes into account the iterative influence of flow corrections between loops which have common pipes. In HC method, an initial flow distribution,
which satisfies flow continuity at nodes, is assumed. Nevertheless, the simultaneous loop flow correction method converges more rapidly to the true solution than the loop by loop Hardy Cross method.

The first order estimates of the loop flow corrections, which would reduce the loop out-ofbalance heads to zero, are found from the following Newton - Raphson approximations.


Twenty Four (24) Pipeline Network Case Study


Figure 2: (Network Diagram Of The Study Area)

## The Network Diagram



Figure 3: (Network Diagram With Pipe Length, Discharge, Nodes, And The Loops

The figure above show the network diagram of the study area, showing the nodes, the pipes, the loops, and the discharge in all the pipes involved. The network has sixteen fixed grade nodes ( $N F=16$ ), and twenty four pipes ( $\mathrm{NP}=24$ ) which are connected to form nine loop ( $N L=9$ ) and nine path (or pseudo loop) ( $N F-l=9$ ). The geometric data for
the network is shown in Table 1. In order to simplify the network/demands at the nodes, pumps and pressure reducing valves were not included. This enabled an exact solution for flows in all pipes and heads at the nodes to be determined analytically. Results from simultaneous loop flow techniques can then be obtained.

Table 1: (Table values)

| Pipe | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{7}$ | $\mathbf{8}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| D | 0.30 | 0.15 | 0.30 | 0.30 | 0.20 | 0.20 | 0.30 | 0.60 |
| L | 700 | 300 | 400 | 350 | 350 | 200 | 300 | 500 |
| K | 576 | 7,901 | 329.22 | 288.06 | $2,187.5$ | 1,250 | 246.91 | 38.58 |
| $\mathrm{Q}_{0}$ | 0.40 | 0.12 | 0.695 | 0.04 | 0.575 | 0.28 | 0.30 | 0.20 |
| hL | 92.18 | 113.77 | 159.02 | 0.46 | 723.24 | 98.00 | 22.22 | 1.54 |
| $\mathrm{Nh} / \mathrm{Q}$ | 460.9 | 1,896 | 457 | 23 | 2,512 | 700 | 148 | 15 |


| Pipe | $\mathbf{9}$ | $\mathbf{1 0}$ | $\mathbf{1 1}$ | $\mathbf{1 2}$ | $\mathbf{1 3}$ | $\mathbf{1 4}$ | $\mathbf{1 5}$ | $\mathbf{1 6}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| D | 0.15 | 0.15 | 0.15 | 0.15 | 0.15 | 0.30 | 0.30 | 0.15 |
| L | 200 | 200 | 200 | 200 | 200 | 350 | 800 | 200 |
| K $^{2}$ | $5,267.5$ | $5,267.5$ | $5,267.5$ | $5,267.5$ | $5,267.5$ | 288.06 | 658.43 | $5,267.49$ |
| $\mathrm{Q}_{0}$ | 0.10 | 0.14 | 0.14 | 0.175 | 0.18 | 0.10 | 0.06 | 0.175 |
| hL | 253.12 | 52.67 | 103.24 | 161.32 | 170.67 | 2.88 | 2.37 | 161.32 |
| $\mathrm{Nh} / \mathrm{Q}$ | 1,687 | 1,053 | 1,476 | 1,844 | 1,896 | 58 | 79 | 1,855 |
| Pipe | $\mathbf{1 7}$ | $\mathbf{1 8}$ | $\mathbf{1 9}$ | $\mathbf{2 0}$ | $\mathbf{2 1}$ | $\mathbf{2 2}$ | $\mathbf{2 3}$ | $\mathbf{2 4}$ |
| D | 0.15 | 0.15 | 0.15 | 0.30 | 0.30 | 0.30 | 0.15 | 0.30 |
| L | 200 | 300 | 200 | 350 | 500 | 600 | 300 | 600 |
| K | $5,267.4$ <br> 9 | $7,901.23$ | $5,267.49$ | 288.06 | 411.52 | 493.83 | $7,901.2$ | 493.83 |
| $\mathrm{Q}_{0}$ | 0.14 | 0.28 | 0.21 | 0.415 | 0.04 | 0.625 | 0.21 | 0.42 |
| hL | 103.24 | 619.45 | 232.30 | 49.61 | 0.66 | 192.90 | 348.44 | 87.11 |
| $\mathrm{Nh} / \mathrm{Q}$ | 737 | 4,425 | 2,212 | 239 | 33 | 617 | 3,318 | 415 |

Here, $\mathrm{h}_{\mathrm{L}}=\mathrm{KQ}^{2}$ where K (Weisbach constant $)=$ 8 fL $\frac{\pi^{2} \mathrm{gD}}{}{ }^{5}$

$f$ is Weisbach friction factor which is approximately 0.0242 and g is gravitational constant taken to be 9.81 .

This set of linear simultaneous equation in $\Delta$ can be written in matrix form as follows:-

LOOP 1
From the Network diagram in figure 3, the Head Loss in each loop can be determined thus;
$\left[\begin{array}{l}\Delta q_{1} \\ \Delta q_{2} \\ \end{array}\right]=\left(\begin{array}{l}-\mathrm{h}_{1} \\ -h_{2} \\ \end{array}\right]$
$\Delta q_{n}$
$-\mathrm{h}_{\mathrm{n}}$
$\mathrm{K}_{7} \mathrm{Q}^{2}{ }_{7}+\mathrm{K}_{8} \mathrm{Q}_{8}{ }^{2}-\mathrm{K}_{6} \mathrm{Q}_{6}{ }^{2}-\mathrm{K}_{1} \mathrm{Q}_{1}{ }^{2}=\mathrm{H}_{1}-\mathrm{H}_{1}$ $=0$

## LOOP 2

$\mathrm{K}_{6} \mathrm{Q}^{2}{ }_{6}+\mathrm{K}_{9} \mathrm{Q}_{9}{ }^{2}+\mathrm{K}_{5} \mathrm{Q}_{5}{ }^{2}-\mathrm{K}_{2} \mathrm{Q}_{2}{ }^{2}=\mathrm{H}_{2}-\mathrm{H}_{2}$ $=0$

LOOP 3
$-\mathrm{K}_{5} \mathrm{Q}^{2}{ }_{5}-\mathrm{K}_{10} \mathrm{Q}_{10}{ }^{2}+\mathrm{K}_{4} \mathrm{Q}^{2}{ }_{4}-\mathrm{K}_{3} \mathrm{Q}^{2}{ }_{3}=\mathrm{H}_{3}-\mathrm{H}_{3}$ $=0$

LOOP 4
$\mathrm{K}_{10} \mathrm{Q}_{10}{ }^{2}-\mathrm{K}_{12} \mathrm{Q}_{12}^{2}-\mathrm{K}_{17} \mathrm{Q}_{17}^{2}+\mathrm{K}_{11} \mathrm{Q}_{11}=\mathrm{H}_{5}-\mathrm{H}_{5}$ $=0$

## LOOP 5

$-\mathrm{K}_{9} \mathrm{Q}_{9}{ }^{2}+\mathrm{K}_{13} \mathrm{Q}_{13}{ }^{2}-\mathrm{K}_{16} \mathrm{Q}^{2}{ }_{16}+\mathrm{K}_{12} \mathrm{Q}_{12}{ }^{2}=\mathrm{H}_{6}-\mathrm{H}_{6}$ $=0$

LOOP 6
$-\mathrm{K}_{8} \mathrm{Q}_{8}{ }^{2}+\mathrm{K}_{14} \mathrm{Q}_{14}{ }^{2}+\mathrm{K}_{15} \mathrm{Q}_{15}{ }^{2}-\mathrm{K}_{13} \mathrm{Q}^{2}{ }_{13}=\mathrm{H}_{7}-\mathrm{H}_{7}$ $=0$
$J \Delta Q=-H$
LOOP 7
$\mathrm{K}_{21} \mathrm{Q}_{21}{ }^{2}-\mathrm{K}_{22} \mathrm{Q}_{22}{ }^{2}-\mathrm{K}_{20} \mathrm{Q}_{20}{ }^{2}-\mathrm{K}_{15} \mathrm{Q}_{15}{ }^{2}=\mathrm{H}_{9}-\mathrm{H}_{9}$
LOOP 8
$\mathrm{K}_{20} \mathrm{Q}_{20}{ }^{2}-\mathrm{K}_{23} \mathrm{Q}_{23}{ }^{2}+\mathrm{K}_{19}{ }^{2}+\mathrm{K}_{16} \mathrm{Q}^{2}{ }_{16}=\mathrm{H}_{10}-\mathrm{H}_{10}$ $=0$

LOOP 9
$-\mathrm{K}_{19} \mathrm{Q}_{19}{ }^{2}-\mathrm{K}_{24} \mathrm{Q}_{24}{ }^{2}+\mathrm{K}_{18} \mathrm{Q}_{18}{ }^{2}+\mathrm{K}_{17} \mathrm{Q}_{17}{ }^{2}=\mathrm{H}_{11}-\mathrm{H}_{11}$ $=0$

This gives the first computation for the head loss in loops.

| $22.22+1.54-98-92.18$ | $=-166.42$ |
| :--- | :--- |
| $98+253.12+723.24-113.77$ | $=960.59$ |
| $-723.24-52.67+0.46-159.02$ | $=-934.47$ |
| $52.67-161.32-103.24+103.24$ | $=-108.65$ |
| $-253.12+170.67-161.32+161.32$ | $=-82.45$ |
| $-1.54+2.88+2.88+2.37-170.67$ | $=-166.96$ |
| $0.66-192.90-49.61-2.37$ | $=-244.22$ |
| $49.61-348.44+232.30+161.32$ | $=94.79$ |
| $-232.30-87.11+619.45+103.24$ | $=$ |



The first matrix formation that is drawn from the table The matrix values above (table 1) is shown in matrix 1 .

$$
\begin{aligned}
& \mathrm{J}_{1-} \\
& \left(\begin{array}{lllllllll}
5,472 & -700 & 0 & 0 & 0 & -15 & 0 & 0 & 0 \\
-700 & 6,798 & -2,515 & 0 & -1,687 & 0 & 0 & 0 & 0 \\
0 & -2,515 & 4,048 & -1,052 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & -1,053 & 5,110 & -1,844 & 0 & 0 & 0 & -737 \\
0 & -1,687 & 0 & -1,844 & 7,271 & -1,896 & 0 & -1,844 & 0 \\
-15 & 0 & 0 & 0 & -1,896 & 2,048 & -79 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & -79 & 968 & -239 & 0 \\
0 & 0 & 0 & 0 & -1,844 & 0 & -239 & 7,613 & -2,212 \\
0 & 0 & 0 & -737 & 0 & 0 & 0 & -2,212 & 7,789
\end{array}\right)\left[\begin{array}{l}
\Delta q_{t} \\
\Delta q_{2} \\
\Delta q_{3} \\
\Delta q_{4} \\
\Delta q_{5} \\
\Delta q_{6} \\
\Delta q_{7} \\
\Delta q_{s} \\
\Delta q_{9}
\end{array}\right]=\left(\begin{array}{l}
166.42 \\
-960.59 \\
934.47 \\
108.65 \\
82.45 \\
166.96 \\
244.22 \\
-94.79 \\
-403.28
\end{array}\right) \\
& \Delta q_{1} \quad=0.0259, \quad \Delta q_{2}=-0.0384, \quad \Delta q_{3}=0.2290, \quad \Delta q_{4}=0.0846 \\
& \Delta q_{s} \quad=0.0624 \quad \Delta q_{6}=0.1497, \quad \Delta q_{7}=0.2640, \quad \Delta q_{3}=-0.0019 \quad \Delta q_{9}=-1.0443
\end{aligned}
$$

Matrix 1
First iteration tables - Table 2

|  | Pipe | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | 7 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | Pipe | $\mathbf{9}$ | $\mathbf{1 0}$ | $\mathbf{1 1}$ | $\mathbf{1 2}$ | $\mathbf{1 3}$ | $\mathbf{1 4}$ | $\mathbf{1 5}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathbf{1 6}$ |  |  |  |  |  |  |  |  |
| $\mathrm{Q}_{0}$ | 0.1992 | - | 0.2246 | 0.1528 | 0.0927 | 0.2597 | - | 0.1107 |
|  |  | 0.0444 |  |  |  |  | 0.054 |  |
| $\mathrm{~h}_{\mathrm{L}}$ | 111.60 | -10.38 | 265.72 | 122.98 | 45.27 | 17.96 | -1.94 | 64.55 |
| $\mathrm{nh} / \mathrm{Q}$ | $1,120.48$ | 467.57 | $2,366.16$ | $1,609.69$ | 976.70 | 143.85 | 71.45 | $1,166.21$ |


|  | Pipe | $\mathbf{1 7}$ | $\mathbf{1 8}$ | $\mathbf{1 9}$ | $\mathbf{2 0}$ | $\mathbf{2 1}$ | $\mathbf{2 2}$ | $\mathbf{2 3}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{Q}_{0}$ | 0.0111 | 0.2357 | 0.2524 | 0.1491 | 0.3040 | 0.3610 | 0.2119 | 0.4643 |
| $\mathrm{~h}_{\mathrm{L}}$ | 0.649 | 438.95 | 335.57 | 6.40 | 38.03 | 64.36 | 354.78 | 106.46 |
| $\mathrm{nh} / \mathrm{Q}$ | 116.94 | $3,724.65$ | $2,659.03$ | 85.85 | 250.20 | 356.57 | $3,348.56$ | 458.58 |

Computation of $Q^{(i)}$ ( from the 1 st iteration)
$\mathrm{Q}_{1}=\mathrm{Q}_{1}{ }^{(0)}-\Delta \mathrm{QL}_{1}=0.40-0258=0.3741$
$\mathrm{Q}_{2}=\mathrm{Q}_{2}{ }^{(0)}-\Delta \mathrm{QL}_{2}=0.12-(-0.0384)=0.1584$
$\mathrm{Q}_{3}=\mathrm{Q}_{3}{ }^{(0)}-\Delta \mathrm{QL}_{3}=0.695-0.2290=0.4660$
$\mathrm{Q}_{4}=\mathrm{Q}_{4}{ }^{(0)}-\Delta \mathrm{QL}_{3}=0.04+0.2290=0.2690$
$\mathrm{Q}_{5}=\mathrm{Q}_{5}{ }^{(0)}-\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{3}=0.575-0.0384-0.2290$ $=0.3076$
$\mathrm{Q}_{6}=\mathrm{Q}_{6}{ }^{(0)}-\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{2}=0.28-0.0259-0.0384=$ 0.2157
$\mathrm{Q}_{7}=\mathrm{Q}_{7}{ }^{(0)}+\Delta \mathrm{QL}_{1}=0.3+0.0259=0.3259$
$\mathrm{Q}_{8}=\mathrm{Q}_{8}{ }^{(0)}+\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{6}=0.2+0.0259-0.1497=$ 0.0762
$\mathrm{Q}_{9}=\mathrm{Q}_{9}{ }^{(0)}+\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{5}=0.3-0.0384-0.0624=$ 0.1992
$\mathrm{Q}_{10}=\mathrm{Q}_{10}{ }^{(0)}-\Delta \mathrm{QL}_{10}+\Delta \mathrm{QL}_{4}=0.10-0.2290+$ $0.0846=-0.0444$
$\mathrm{Q}_{11}=\mathrm{Q}_{11}{ }^{(0)}+\Delta \mathrm{QL}_{3}+\Delta \mathrm{QL}_{4}=0.14+0.0846=$ 0.2246
$\mathrm{Q}_{12}=\mathrm{Q}_{12}{ }^{(0)}-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{5}=0.175-0.0846+$
$0.0624=0.1528$
$\mathrm{Q}_{13}=\mathrm{Q}_{13}{ }^{(0)}+\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{6}=0.18+0.0624-$
$0.1497=0.0927$
$\mathrm{Q}_{14}=\mathrm{Q}_{14}{ }^{(0)}+\Delta \mathrm{QL}_{6}=0.1+0.1497=0.2497$
$\mathrm{Q}_{15}=\mathrm{Q}_{15}{ }^{(0)}+\Delta \mathrm{QL}_{6}-\Delta \mathrm{QL}_{7}=0.06+0.1497-$
$0.2640=0.0543$
$\mathrm{Q}_{16}=\mathrm{Q}_{16}{ }^{(0)}-\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{8}=0.175-0.0624-$ $0.0019=0.1107$
$\mathrm{Q}_{17}=\mathrm{Q}_{17}{ }^{(0)}-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{9}=0.14-0.0846-$ $0.0443=0.0111$
$\mathrm{Q}_{18}=\mathrm{Q}_{18}{ }^{(0)}+\Delta \mathrm{QL}_{9}=0.28-0.0443=0.2357$
$\mathrm{Q}_{19}=\mathrm{Q}_{19}{ }^{(0)}-\Delta \mathrm{QL}_{8}+\Delta \mathrm{QL}_{9}=0.21-0.0019+0.0443$ $=0.2524$
$\mathrm{Q}_{20}=\mathrm{Q}_{20}{ }^{(0)}-\Delta \mathrm{QL}_{7}+\Delta \mathrm{QL}_{8}=0.415-0.2640-$ $0.0019=0.1491$
$\mathrm{Q}_{21}=\mathrm{Q}_{21}{ }^{(0)}+\Delta \mathrm{QL}_{7}=0.04+02640=0.3040$
$\mathrm{Q}_{22}=\mathrm{Q}_{22}{ }^{(0)}-\Delta \mathrm{QL}_{7}=0.625-0.2640=0.3610$
$\mathrm{Q}_{23}=\mathrm{Q}_{23}{ }^{(0)}-\Delta \mathrm{QL}_{8}=0.21+0.0019=0.2119$
$\mathrm{Q}_{24}=\mathrm{Q}_{24}{ }^{(0)}-\Delta \mathrm{QL}_{9}=0.42+00.21+0.0443=0.4643$
The second computations for the head loss in loops.
$\mathrm{h}_{1}=26.22+0.224-58.16-265.72=$

- 297.43
$\mathrm{h}_{2}=58.16+111.60+206.98-198.25=$ 178.49
$\mathrm{h}_{3} \quad=\quad-206.98+10.38+20.84-71.49$
$-247.25$
$\mathrm{h}_{4}=-10.38-122.98-0.649+265.72=\mathrm{h}_{7}=38.03-64.36-6.40+1.94 \quad=$ 131.71

$$
-30.79
$$

$$
\mathrm{h}_{5}=-111.60+45.27-64.55+122.98=\mathrm{h}_{8}=6.40-354.78+335.57+64.55=
$$

$-7.90$ 51.74
$\mathrm{h}_{6}=-0.224+17.96-1.94-45.27 \quad=\quad \mathrm{h}_{9} \quad=\quad-335.57-106.46+438.95+0.649=$ - 29.47 $-2.43$

The second matrix formation that is drawn from the first iteration table (table 2 ) is shown in matrix 2 below.

## Matrix 2

## $2^{\text {nd }}$ Iteration Tables - Table 3

| Pipe | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{7}$ | $\mathbf{8}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{Q}\left(^{2}\right)$ | 0.1001 | 0.1326 | 0.3448 | 0.3902 | 0.2122 | -0.032 | 0.5999 | 0.3144 |
| hL | 5.77 | 138.93 | 39.14 | 43.86 | 98.50 | -1.32 | 88.86 | 3.81 |
| $\mathrm{Nhl} / \mathrm{Q}$ | 115.28 | $2,095.48$ | 227.03 | 225.61 | 928.37 | 81.23 | 296.25 | 24.24 |


| Pipe | $\mathbf{9}$ | $\mathbf{1 0}$ | $\mathbf{1 1}$ | $\mathbf{1 2}$ | $\mathbf{1 3}$ | $\mathbf{1 4}$ | $\mathbf{1 5}$ | $\mathbf{1 6}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :---: |
| $\mathrm{Q}\left(^{2}\right)$ | 0.2161 | -0.1790 | 0.2112 | 0.1751 | 0.0658 | 0.2855 | - | 0.0958 |
| hL | 131,34 | -168.78 | 234.96 | 161.50 | 22.81 | 23.48 | -2.49 | 48.34 |
| Nhl/Q | $1,215.55$ | $1,885.81$ | 2,2251 | 1844.66 | 693.31 | 164.48 | 80.98 | $1,009.19$ |


| pipe | $\mathbf{7}$ | $\mathbf{8}$ | $\mathbf{9}$ | $\mathbf{2 0}$ | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\left(^{2}\right)$ | .0223 | .2335 | .2486 | 1001 | 0. | .3470 | .3180 | .2179 |
| L | .62 | 30.79 | 25.54 | 89 | 2. | 9.55 | 9.94 | 75.15 |
|  |  |  |  | 57 |  |  |  | 07.47 |

$$
\begin{aligned}
& \mathbf{J}_{2}=\left(\begin{array}{lllllllll} 
& 0 & 0 & -5.88 & 0 & 0 & 0 & 0 \\
1,137.12 & -539.27 & 0 & 0 & -1.120 .48 & 0 & 0 & 0 & 0 \\
-539.27 & 5,508.68 & -1345.77 & 0 & 0 & 0 & 0 & 0 \\
0 & -1345.72 & 2,275.11 & -467.57 & 0 & 0 & 0 & -116.97 \\
0 & 0 & -467.57 & 4,560.36 & -1,609.69 & 0 & 0 \\
0 & -1,120.48 & 0 & -1,609.69 & 4,873.08 & -976.70 & 0 & -1,166.21 & 0 \\
-5.88 & 0 & 0 & 0 & -976.70 & 1,197.88 & -71.45 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & -71.457 & 64.07 & -85.85 & 0 \\
0 & 0 & 0 & 0 & -1,166.21 & 0 & -85.85 & 7,259.65 & -2,659.03 \\
e & 0 & 0 & -116.94 & 0 & 0 & 0 & -2,659.03 & 6,959.2
\end{array}\right)\left(\begin{array}{l} 
\\
0
\end{array}\right)\left(\begin{array}{l}
\Delta q_{1} \\
\Delta q_{2} \\
\Delta q_{3} \\
\Delta q_{4} \\
\Delta q_{5} \\
\Delta q_{6} \\
\Delta q_{7} \\
\Delta q_{3} \\
\Delta q_{0}
\end{array}\right)=\left(\begin{array}{l} 
\\
297.436 \\
-178.49 \\
247.25 \\
-13.711 \\
7.90 \\
29.474 \\
30.79 \\
-51.74 \\
2.431
\end{array}\right) \\
& \Delta q_{\mathrm{t}}=0.2740, \quad \Delta q_{2}=0.0258 ; \quad \Delta q_{3} \quad=0.1212, \quad \Delta q_{4}=-0.0134 \\
& \Delta q_{s}=0.0089, \quad \Delta q_{s}=0.0358 \quad \Delta q_{7}=0.0430 \quad \Delta q_{\mathrm{s}}=-0.0060 \quad \Delta q_{p}=-0.0022 .
\end{aligned}
$$

| $\mathrm{hl} / \mathrm{Q}$ | 34.98 | , 689.85 | , 618.99 | .74 | 85.59 | 14.09 | .443 .32 | 60.75 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Computation of $Q^{(2)}\left(\right.$ from $2^{\text {nd }}$ iteration)
$\mathrm{Q}_{1}=\mathrm{Q}^{(1)}-\Delta \mathrm{QL}_{1}=0.3741-02740=0.1001$
$\mathrm{Q}_{2}=\mathrm{Q}_{2}{ }^{(1)}-\Delta \mathrm{QL}_{2}=0.1584-0.0258=0.1326$
$\mathrm{Q}_{3}=\mathrm{Q}_{3}{ }^{(1)}-\Delta \mathrm{QL}_{3}=0.4660-0.1212=0.3448$
$\mathrm{Q}_{4}=\mathrm{Q}_{4}{ }^{(1)}-\Delta \mathrm{QL}_{3}=0.2690+0.1212=0.3902$
$\mathrm{Q}_{5}=\mathrm{Q}_{5}{ }^{(1)}-\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{3}=0.3076+0.0258-0.1212$ $=0.2122$
$\mathrm{Q}_{6}=\mathrm{Q}_{6}{ }^{(1)}-\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{2}=0.2157-0.2740+0.0258$ $=-0.0325$
$\mathrm{Q}_{7}=\mathrm{Q}_{7}{ }^{(1)}+\Delta \mathrm{QL}_{1}=0.3259+0.2740=0.5999$
$\mathrm{Q}_{8}=\mathrm{Q}_{8}{ }^{(1)}+\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{6}=0.0762+0.2740-$ $0.0358=0.3144$
$\mathrm{Q}_{9}=\mathrm{Q}_{9}{ }^{(1)}+\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{5}=0.1992+0.0258-$ $0.0089=0.2161$
$\mathrm{Q}_{10=\mathrm{Q} 10(1)}-\Delta \mathrm{QL}_{3}+\Delta \mathrm{QL}_{4}=0.0444-0.1212-$ $0.0134=-0.1790$
$\mathrm{Q}_{11}=\mathrm{Q}_{11}{ }^{(1)}+\Delta \mathrm{QL}_{4}=0.2246-0.0134=0.2112$
$\mathrm{Q}_{12}=\mathrm{Q}_{12}{ }^{(1)}-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{5}=0.1528+0.0134+$ $0.0089=0.1751$

The third computation of the head loss in each loop.
$\mathrm{h}_{1}=88.86+3.81+1.3-5.77=88.22$
$\mathrm{h}_{2} \quad=\quad-1.32+131.34+98.50-138.93=$
89.59
$\mathrm{h}_{3}=-98.50+168.78+43.86-39.14=$ $\begin{aligned} & 75.00 \\ & \mathrm{~h}_{4}\end{aligned}=-168.78-161.50-2.62+234.96=$ 97.94
$\mathrm{Q}_{13}=\mathrm{Q}_{13}{ }^{(1)}+\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{6}=0.0927+0.0089-$ $0.0358=0.0658$
$\mathrm{Q}_{14}=\mathrm{Q}_{14}{ }^{(1)}+\Delta \mathrm{QL}_{6}=0.2497+0.0358=0.2855$
$\mathrm{Q}_{15}=\mathrm{Q}_{15}{ }^{(1)}+\Delta \mathrm{QL}_{6}-\Delta \mathrm{QL}_{7}=-0.0543+0.0358-$
$0.0430=-06150$
$\mathrm{Q}_{16}=\mathrm{Q}_{16}{ }^{(1)}-\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{8}=0.1107-0.0089-$
$0.0060=0.0958$
$\mathrm{Q}_{17}=\mathrm{Q}_{17}{ }^{(1)}-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{9}=0.0111+0.0134-$ $0.0022=0.0223$
$\mathrm{Q}_{18}=\mathrm{Q}_{18}{ }^{(1)}+\Delta \mathrm{QL}_{9}=0.2357-0.0022=0.2335$
$\mathrm{Q}_{19}=\mathrm{Q}_{19}{ }^{(1)}-\Delta \mathrm{QL}_{8}+\Delta \mathrm{QL}_{9}=0.2524-0.0060+$ $0.0022=0.2486$
$\mathrm{Q}_{20}=\mathrm{Q}_{20}{ }^{(1)}-\Delta \mathrm{QL}_{7}+\Delta \mathrm{QL}_{8}=0.1491-0.0430-$
$0.0060=0.1001$
$\mathrm{Q}_{21}=\mathrm{Q}_{21}{ }^{(1)}+\Delta \mathrm{QL}_{7}=0.3040+0.0430=0.3470$
$\mathrm{Q}_{22}=\mathrm{Q}_{22}{ }^{(1)}-\Delta \mathrm{QL}_{7}=0.3610-0.0430=0.3180$
$\mathrm{Q}_{23}=\mathrm{Q}_{23}{ }^{(1)}-\Delta \mathrm{QL}_{8}=0.2119+0.0060=0.2179$
$\mathrm{Q}_{24}=\mathrm{Q}_{24}{ }^{(1)}-\Delta \mathrm{QL}_{9}=0.4643+0.0022=0.4665$

| $\mathrm{h}_{1}$ | = | $88.86+3.81+1.3-5.77=88.22$ | 4.63 |  |
| :---: | :---: | :---: | :---: | :---: |
| $\mathrm{h}_{2}$ | $=$ | $-1.32+131.34+98.50-138.93=$ | $\mathrm{h}_{6}$ | $-3.81+23.48-2.49-22.81=-5.63$ |
| 89.59 |  |  | $\mathrm{h}_{7}$ | $49.55-49.94-2.89+2.49$ |
| $\mathrm{h}_{3}$ | $=$ | $-98.50+168.78+43.86-39.14$ | 0.08 |  |
| 75.00 |  |  | $\mathrm{h}_{8}$ | $2.89-375.15+325.54+48.34=1.62$ |
| $\mathrm{h}_{4}$ |  | $-168.78-161.50-2.62+234.96$ | $\mathrm{h}_{9}$ | $-325.54-107.47+430.79+2.62$ |
| 97.94 |  |  | $=0.40$ |  |

The third matrix formation that is drawn from the second iteration table (table 3 ) is shown in matrix 3


Matrix 3
$3^{\text {rd }}$ Iteration Tables - Table 4

| Pipe | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{7}$ | $\mathbf{8}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{Q}\left(^{2}\right)$ | 0.2762 | 0.1656 | 0.3749 | 0.3601 | 0.2093 | 0.1106 | 0.4238 | 0.1433 |
| hL | 43.95 | 216.68 | 46.27 | 37.35 | 95.83 | 15.29 | 44.35 | 0.79 |
| $\mathrm{Nhl} / \mathrm{Q}$ | 318.25 | $2,616.91$ | 246.84 | 207.44 | 915.72 | 276.49 | 209.30 | 11.03 |


| Pipe | $\mathbf{9}$ | $\mathbf{1 0}$ | $\mathbf{1 1}$ | $\mathbf{1 2}$ | $\mathbf{1 3}$ | $\mathbf{1 4}$ | $\mathbf{1 5}$ | $\mathbf{1 6}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{Q}\left({ }^{2}\right)$ | 0.1920 | -0.1449 | 0.2152 | 0.1622 | 0.0619 | 0.2805 | - <br> 0.0669 | 0.1030 |
| hL | 103.68 | -110.60 | 243.94 | 138.58 | 20.18 | 22.66 | -2.95 | 55.88 |
| Nhl/Q | $1,080.00$ | $1,526.57$ | $2,267.10$ | $1,708.75$ | 652.02 | 161.57 | 88.19 | $1,085.05$ |


| Pipe | $\mathbf{1 7}$ | $\mathbf{1 8}$ | $\mathbf{1 9}$ | $\mathbf{2 0}$ | $\mathbf{2 1}$ | $\mathbf{2 2}$ |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{Q}_{0}$ | 0.0177 | 0.2329 | 0.2475 | 0.0980 | 0.3474 | 0.3176 | 0.2196 |
| hL | 1.65 | 428.58 | 322.67 | 2.77 | 49.67 | 49.81 | 381.03 |
| $\mathrm{nh} / \mathrm{Q}$ | 186.44 | $3,680.38$ | 2.60 .43 | 56.53 | 285.95 | 313.66 | $3,470.22$ |

## Computation of Q3 (from $3^{\text {rd }}$ iteration)

$\mathrm{Q}_{1}=\mathrm{Q}_{1}\left({ }^{2}\right)-\Delta \mathrm{QL}_{1}=0.1001+0.1761=0.2762$
$\left.\mathrm{Q}_{2}=\mathrm{Q}_{2}{ }^{(2}\right)-\Delta \mathrm{QL}_{2}=0.1326+0.0330=0.1656$
$\mathrm{Q}_{3}=\mathrm{Q}_{3}\left({ }^{2}\right)-\Delta \mathrm{QL}_{3}=0.3448+0.0301=0.3749$
$\mathrm{Q}_{4}=\mathrm{Q}_{4}\left({ }^{2}\right)-\Delta \mathrm{QL}_{3}=0.3902-0.0301=0.3601$
$\mathrm{Q}_{5}=\mathrm{Q}_{5}\left({ }^{2}\right)-\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{3}=0.2122-0.0330$ $0.0301=0.2093$
$\mathrm{Q}_{6}=\mathrm{Q}_{6}\left({ }^{2}\right)-\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{2}=0.0325+0.1761-00040=-0.1449$
$0.0330=0.1106$
$\mathrm{Q}_{7}=\mathrm{Q}_{7}\left({ }^{2}\right)+\Delta \mathrm{QL}_{1}=0.5999-0.1761=0.4238$
$\mathrm{Q}_{8}=\mathrm{Q}_{8}\left({ }^{2}\right)+\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{6}=0.3144-0.1761+$
$0.0050=0.1433$
$\mathrm{Q}_{9}=\mathrm{Q}_{9}\left({ }^{2}\right)+\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{5}=0.2161-0.0330+$
$+\quad 0.0089=0.1920$
$\mathrm{Q}_{10}=\mathrm{Q}_{10}\left({ }^{2}\right)-\Delta \mathrm{QL}_{3}+\Delta \mathrm{QL}_{4}=-0.1790+0.0301+$
$\mathrm{Q}_{11}=\mathrm{Q}_{11}\left({ }^{2}\right)+\Delta \mathrm{QL}_{4}=0.2112+0.0040=0.2152$
$\mathrm{Q}_{12}=\mathrm{Q}_{12}\left({ }^{2}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{5}=0.1751-0.0040-\mathrm{Q}_{18}=\mathrm{Q}_{18}\left({ }^{2}\right)+\Delta \mathrm{QL}_{9}=02335-0.0006=0.2329$
$0.0089=0.1622$
$\mathrm{Q}_{13}=\mathrm{Q}_{13}\left({ }^{2}\right)+\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{6}=0.0658-0.0089+$ $0.0050=0.0619$
$\mathrm{Q}_{14}=\mathrm{Q}_{14}\left({ }^{2}\right)+\Delta \mathrm{QL}_{6}=0.2855-0.0050=0.2805$
$\left.\mathrm{Q}_{15}=\mathrm{Q}_{15}{ }^{( }{ }^{2}\right)+\Delta \mathrm{QL}_{6}-\Delta \mathrm{QL}_{7}=-0.0615-0.0050-$
$0.0004=-0.0669$
$\left.\mathrm{Q}_{16}=\mathrm{Q}_{16}{ }^{(2}\right)-\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{8}=0.0958+0.0089-$ $0.0017=0.1030$
$\left.\mathrm{Q}_{17}=\mathrm{Q}_{17}{ }^{(2}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{9}=0.0223-0.0040-$ $0.0006=0.0177$
$\mathrm{Q}_{19}=\mathrm{Q}_{19}\left({ }^{2}\right)-\Delta \mathrm{QL}_{8}+\Delta \mathrm{QL}_{9}=0.2486-0.0017+$ $0.0006=0.2475$
$\left.\mathrm{Q}_{20}=\mathrm{Q}_{20}{ }^{( }{ }^{2}\right)-\Delta \mathrm{QL}_{7}+\Delta \mathrm{QL}_{8}=0.1001-0.0004-$ $0.0017=0.0980$
$\mathrm{Q}_{21}=\mathrm{Q}_{21}\left({ }^{2}\right)+\Delta \mathrm{QL}_{7}=0.3470+0.0004=0.3474$
$\mathrm{Q}_{22}=\mathrm{Q}_{22}\left({ }^{2}\right)-\Delta \mathrm{QL}_{7}=0.3180-0.0004=0.3176$
$\mathrm{Q}_{23}=\mathrm{Q}_{23}\left({ }^{2}\right)-\Delta \mathrm{QL}_{8}=0.2179+0.0017=0.2196$
$\mathrm{Q}_{24}=\mathrm{Q}_{24}\left({ }^{2}\right)-\Delta \mathrm{QL}_{9}=0.4665+0.0006=0.4671$

The fourth head loss determination in each loop. $\quad h_{5} \quad=\quad-103.68+20.18-55.88+138.58$

| $\mathrm{h}_{1}$ | $=$ | $44.35+0.79-15.29-43.95$ |  | $=$ | $-0.79+22.6-2.95-20.18$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $=-14.1$ |  |  | $=-1.26$ |  |  |
| $\begin{aligned} & \mathrm{h}_{2} \\ & =-1.88 \end{aligned}$ | $=$ | $15.29+103.68+95.83-216.68$ | $\begin{aligned} & \mathrm{h}_{7} \\ & =0.04 \end{aligned}$ | $=$ | $49.67-49.81-2.77+2.95$ |
| $\mathrm{h}_{3}$ | = | $-95.83+110.60+37.35-46.27$ | $\mathrm{h}_{8}$ | $=$ | $2.77-281.03+322.67+55.88$ |
| $=5.85$ |  |  | $=0.29$ |  |  |
| $\begin{aligned} & \mathrm{h}_{4} \\ & =-6.89 \end{aligned}$ | $=$ | $-110.60-138.58-1.65+243.94$ | $\mathrm{h}_{9}$ $=-14.47$ | $=$ | $-322.67-122.03+428.58+1.65$ |

The fourth matrix formation that is drawn from the third iteration table (table 4) is shown in matrix 4


## Matrix 4

$4^{\text {th }}$ Iteration Tables - Table 5

| Pipe | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{7}$ | $\mathbf{8}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\left.\mathrm{Q}^{4}\right)$ | 0.2583 | 0.1639 | 0.3755 | 0.3595 | 0.2116 | 0.0944 | 0.4417 | 0.1582 |
| hL | 38.44 | 212.25 | 46.42 | 37.23 | 97.94 | 11.14 | 48.17 | 0.97 |
| $\mathrm{Nhl} / \mathrm{Q}$ | 297.64 | $2,589,99$ | 247.24 | 207.12 | 925.71 | 236.02 | 218.11 | 12.26 |


| Pipe | $\mathbf{9}$ | $\mathbf{1 0}$ | $\mathbf{1 1}$ | $\mathbf{1 2}$ | $\mathbf{1 3}$ | $\mathbf{1 4}$ | $\mathbf{1 5}$ | $\mathbf{1 6}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\left.\mathrm{Q}^{4}\right)$ | 0.1917 | -0.1426 | 0.2169 | 0.1625 | 0.0609 | 0.2835 | -0.064 | 0.1022 |
| hL | 103.36 | -107.11 | 247.81 | 139.09 | 19.54 | 23.15 | -2.72 | 55.02 |
| $\mathrm{Nhl} / \mathrm{Q}$ | $1,078.3$ <br> 5 | $1,502.24$ | $2,285.02$ | $1,711.88$ | 641.71 | 163.32 | 84.60 | $1,076.71$ |


| Pipe | $\mathbf{1 7}$ | $\mathbf{1 8}$ | $\mathbf{1 9}$ | $\mathbf{2 0}$ | $\mathbf{2 1}$ | $\mathbf{2 2}$ | $\mathbf{2 3}$ | $\mathbf{2 4}$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\left.\mathrm{Q}^{4}\right)$ | 0.0186 | 0.2355 | 0.2461 | 0.0988 | 0.3478 | 0.3172 | 0.2184 | 0.4945 |
| hL | 1.82 | 438.20 | 319.03 | 2.81 | 49.78 | 49.69 | 376.88 | 120.76 |
| Nhl/Q | 195.70 | 372.44 | 2592.69 | 56.88 | 286.26 | 313.30 | $3,451.2$ <br> 8 | 488.41 |

## Computation of $\mathrm{Q}^{4}$ ) (from the $4^{\text {th }}$ iteration )

$\mathrm{Q}_{1}=\mathrm{Q}_{1}\left({ }^{3}\right)-\Delta \mathrm{QL}_{1}=0.2762-0.0179=0.2583$
$\mathrm{Q}_{2}=\mathrm{Q}_{2}\left({ }^{3}\right)-\Delta \mathrm{QL}_{2}=0.1656-0.0017=0.1639$
$\mathrm{Q}_{3}=\mathrm{Q}_{3}\left({ }^{3}\right)-\Delta \mathrm{QL}_{3}=0.3749+0.0006=0.3755$
$\mathrm{Q}_{4}=\mathrm{Q}_{4}\left({ }^{3}\right)+\Delta \mathrm{QL}_{3}=0.3601-0.0006=0.3595$
$\mathrm{Q}_{5}=\mathrm{Q}_{5}\left({ }^{3}\right)-\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{3}=0.2093+0.0017+$ $0.0006=0.2116$
$\mathrm{Q}_{6}=\mathrm{Q}_{6}\left({ }^{3}\right)-\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{2}=0.11 .06-0.0179+$ $0.0017=0.0944$
$\mathrm{Q}_{7}=\mathrm{Q}_{7}\left({ }^{3}\right)+\Delta \mathrm{QL}_{1}=0.4238+0.0 .0179=0.4417$
$\mathrm{Q}_{8}=\mathrm{Q}_{8}\left({ }^{3}\right)+\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{6}=0.1433+0.0179$
$0.0030=0.1582$
$\mathrm{Q}_{9}=\mathrm{Q}_{9}\left(\left(^{3}\right)+\Delta \mathrm{DQL}_{2}-\Delta \mathrm{QL}_{5}=0.1920+0.0017-\right.$ $0.0020=0.1917$
$\mathrm{Q}_{10}=\mathrm{Q}_{10}\left({ }^{3}\right)-\Delta \mathrm{QL}_{3}+\Delta \mathrm{QL}_{4}=-0.1449+0.0006+$ $0.0617=-0.1426$
$\left.\mathrm{Q}_{11}=\mathrm{Q}_{11}{ }^{3}\right)+\Delta \mathrm{QL}_{4}=0.2152+0.0017=0.2169$
$\mathrm{Q}_{12}=\mathrm{Q}_{12}\left(^{2}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{5}=0.1622-0.0017+$ $0.0020=0.1625$
$\left.\mathrm{Q}_{13}=\mathrm{Q}_{13}{ }^{3}{ }^{3}\right)+\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{6}=0.0619+0.0020-$ $0.0030=0.0609$
$\mathrm{Q}_{14}=\mathrm{Q}_{14}\left({ }^{3}\right)+\Delta \mathrm{QL}_{6}=0.2805+0.0030=0.2835$
$\mathrm{Q}_{15}=\mathrm{Q}_{15}\left({ }^{3}\right)+\Delta \mathrm{QL}_{6}-\Delta \mathrm{QL}_{7}=-0.0669+0.0030-$ $0.0004=-0.0643$
$\mathrm{Q}_{16}=\mathrm{Q}_{16}\left({ }^{3}\right)-\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{8}=0.1030-0.0020$ $+0.0012=0.1022$
$\mathrm{Q}_{17}=\mathrm{Q}_{17}\left(^{3}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{9}=0.0177-0.0017+$ $0.0026=0.0186$
$\mathrm{Q}_{18}=\mathrm{Q}_{18}\left({ }^{3}\right)+\Delta \mathrm{QL}_{9}=0.2329+0.0026=0.2355$
$\mathrm{Q}_{19}=\mathrm{Q}_{19}\left({ }^{3}\right)+\Delta \mathrm{QL}_{8}+\Delta \mathrm{QL}_{9}=0.2475+0.0012-$ $0.0026=0.2461$
$\left.\mathrm{Q}_{20}=\mathrm{Q}_{20}{ }^{3}{ }^{3}\right)-\Delta \mathrm{QL}_{7}+\Delta \mathrm{QL}_{8}=0.0980-0.0004+$ $0.0012=0.0988$
$\left.\mathrm{Q}_{21}=\mathrm{Q}_{21}{ }^{3}{ }^{3}\right)+\Delta \mathrm{QL}_{7}=0.3474+0.0004=0.3478$
$\mathrm{Q}_{22}=\mathrm{Q}_{22}\left({ }^{3}\right)-\Delta \mathrm{QL}_{7}=0.3176-0.0004=0.3172$
$\mathrm{Q}_{23}=\mathrm{Q}_{23}\left({ }^{3}\right)-\Delta \mathrm{QL}_{8}=0.2196-0.0012=0.2184$
$\mathrm{Q}_{24}=\mathrm{Q}_{24}\left({ }^{3}\right)-\Delta \mathrm{QL}_{9}=0.4971-0.0026=0.4945$

The fifth computation of the head loss in each loop.
$\mathrm{h}_{1}=48.17+0.97-11.14-38.44=-0.44$
$\mathrm{~h}_{2}=11.14+103.36+97.94-212.25=$
0.19
$\mathrm{~h}_{3}=-97.94-107.11+37.23-46.42=-0.02$
$\mathrm{~h}_{4}=-107.11-139.09-1.82+247.81=-0.21$

The fifth matrix formation that is drawn from the fourth iteration table (table 5) is shown in matrix 5


## Computation of $\left.Q \mathbf{(}^{\mathbf{5}}\right)\left(\right.$ from the $5^{\text {th }}$ iteration $)$

$\mathrm{Q}_{1}=\mathrm{Q}_{1}\left({ }^{4}\right)-\Delta \mathrm{QL}_{1}=0.2583-0.5711=-0.3128$
$\mathrm{Q}_{2}=\mathrm{Q}_{2}\left({ }^{4}\right)-\Delta \mathrm{QL}_{2}=0.1639+0.0188=0.1827$
$\mathrm{Q}_{3}=\mathrm{Q}_{3}\left({ }^{4}\right)-\Delta \mathrm{QL}_{3}=0.3755-0.0143=0.3612$
$\mathrm{Q}_{4}=\mathrm{Q}_{4}\left({ }^{4}\right)+\Delta \mathrm{QL}_{3}=0.3595+0.0143=0.3738$
$\mathrm{Q}_{5}=\mathrm{Q}_{5}\left({ }^{4}\right)-\Delta \mathrm{QL}_{2}-\Delta \mathrm{QL}_{3}=0.2116-0.0188-$
$\mathrm{Q}_{6}=\mathrm{Q}_{6}\left({ }^{4}\right)-\Delta \mathrm{QL}_{1}-\Delta \mathrm{QL}_{2}=0.0944-0.5711-$ $0.0188=-0.4955$
$\mathrm{Q}_{7}=\mathrm{Q}_{7}\left({ }^{4}\right)+\Delta \mathrm{QL}_{1}=0.4417+0.0 .5711=1.0128$
$0.0143=0.1785$
$\mathrm{Q}_{10}={ }_{\mathrm{Q} 10}\left({ }^{4}\right)-\Delta \mathrm{QL}_{3}+\Delta \mathrm{QL}_{4}=-0.1426-0.0143+$ $0.0257=-0.1312$
$\mathrm{Q}_{11}=\mathrm{Q}_{11}\left({ }^{4}\right)+\Delta \mathrm{QL}_{4}=0.2169+0.0252=0.2421$
$\left.\mathrm{Q}_{12}=\mathrm{Q}_{12}{ }^{4}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{5}=0.1625-0.0257-$ $0.0452=0.0916$
$\mathrm{Q}_{13}=\mathrm{Q}_{13}\left({ }^{4}\right)+\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{6}=0.0609-0.0452-$ $0.0648=-0.0491$
$\mathrm{Q}_{14}=\mathrm{Q}_{14}\left({ }^{4}\right)+\Delta \mathrm{QL}_{6}=0.2835+0.0648=0.3483$
$\mathrm{Q}_{15}=\mathrm{Q}_{15}\left({ }^{4}\right)+\Delta \mathrm{QL}_{6}-\Delta \mathrm{QL}_{7}=-0.0643+0.0648-$ $0.0060=-0.0055$

## Results and Discussion

The simultaneous loop flow correction analyzer was applied to the water pipeline network in Owerri municipal area, a great reduction of the head loss was achieved in the system. This is pictured in the graphs below.

Graph 1 compares the original head $\operatorname{loss}\left(\mathrm{H}_{0}\right)$ with the corrected one $\left(\mathrm{H}_{\mathrm{f}}\right)$, the original head loss (as calculated from the distribution network from the data collected) shows an irregular head loss across the wax (hops) from the graph, the blue line indicates the original while the red line indicates the corrected head loss. (as calculated after the fifth iteration). The result shows a drastic reduction in head loss which is approximately zero in all the loops involves and the line is regular along the axis $\left(\mathrm{H}_{\mathrm{f}}\right)$.

Graphs 2 and 3 compare the original and the corrected result using the discharge Q and the pipe diameter D. Practically, the rate of flow per minute is directly proportional to the pipe diameter (the longer the diameter, the greater the flow and vise versa). The original flow Q from the data collected shows proportionality with the pipe diameter which is fixed and this in contrary is an indication of bad network design that leads to losses as shown in graph 2. Furthermore, graph 3 pictures the corrected flow rate as it varies with the pipe diameter, thus, this graph indicates a direct proportionality of the flow rate, Q and the pipe diameter, D .
$\left.\mathrm{Q}_{16}=\mathrm{Q}_{16}{ }^{4}\right)-\Delta \mathrm{QL}_{5}-\Delta \mathrm{QL}_{8}=0.1022+0.0452-$
$0.0180=0.1294$
$\left.\mathrm{Q}_{17}=\mathrm{Q}_{17}{ }^{4}\right)-\Delta \mathrm{QL}_{4}+\Delta \mathrm{QL}_{9}=0.0186-0.0257-$
$0.0388=-0.0459$
$\mathrm{Q}_{18}=\mathrm{Q}_{18}\left({ }^{4}\right)+\Delta \mathrm{QL}_{9}=0.2355-0.0388=0.1967$
$\mathrm{Q}_{19}=\mathrm{Q}_{19}\left({ }^{4}\right)+\Delta \mathrm{QL}_{8}+\Delta \mathrm{QL}_{9}=0.2461-0.0180+$
$0.0388=0.2669$
$\mathrm{Q}_{20}=\mathrm{Q}_{20}\left({ }^{4}\right)-\Delta \mathrm{QL}_{7}+\Delta \mathrm{QL}_{8}=0.0988-0.0060-$
$0.0180=0.0868$
$\mathrm{Q}_{21}=\mathrm{Q}_{21}\left(^{4}\right)+\Delta \mathrm{QL}_{7}=0.3478+0.0060=0.3538$
$\mathrm{Q}_{22}=\mathrm{Q}_{22}\left({ }^{4}\right)-\Delta \mathrm{QL}_{7}=0.3172-0.0060=0.3112$
$\left.\mathrm{Q}_{23}=\mathrm{Q}_{23}{ }^{4}\right)-\Delta \mathrm{QL}_{8}=0.2184+0.0180=0.2364$
$\mathrm{Q}_{24}=\mathrm{Q}_{24}\left({ }^{4}\right)-\Delta \mathrm{QL}_{9}=0.4945+0.0388=0.5330$
Graph 4 and 5 compares the pipe length and the head loss along the pipes. From definition, the loss is the energy per unit weight of fluid. In other words, it is the rate of the product of force and the length to the weight of fluid. This definition of head loss indicates that the head loss increases with increase in pipe length. Thus, Graph 4 pictures the original variation of the head loss with the pipe length. The graph lines counter each other, showing the level of non proportionality between the pipe length and head loss while the corrected result produced after the fifth iteration shows the level of proportionality between the two constraints since the line did not counter each other along the axis as shown in graph 5 .

This method (simultaneous loop flow correction) produced an accurate result with a great reduction in the head losses in each loop after the fifth iteration.
In addition to the above achievement, the rate at which the water flows (Q) was observed to be proportional to the pipe diameter as shown in the graph 5. Here, the initial flow rates were compared with the corrected flow rates with respect to the pipe diameters. With the initial discharge, which is not proportional to the diameter, the deformation in pipes will be high. Hence, the simultaneous flow correction technique adopted in this work in analyzing pipeline network of water reticulation system has proven to be the best numerical technique.

## Graphs and Result Curves



Graph 1- The graph of initial $\left(\mathrm{H}_{\mathrm{o}}\right)$ and final head loss $\left(\mathrm{H}_{\mathrm{f}}\right)$ in the nine loops.


Graph 2- The graph that shows the existing discharge, Q/the pipe diameter, D.


Graph 3-The graph that shows the corrected discharge, $\mathrm{Q} /$ the pipe diameter, D .


Graph 4- The graph that shows the initial head loss $\mathrm{H}_{\mathrm{o}}$ /the pipe length, L .

## Conclusion

The simultaneous loop flow correction method were applied to the network distribution that involves twenty four pipes and nine loops, which is one of the highest as regards to the number of pipes analyzed so far.

It has been observed that the existing system has a lot of fluctuations in its head loss. Also, the discharge $(\mathrm{Q})$ is not proportional to the pipe diameter (D). With the application of simultaneous loop flow correction method, the result produced shows a great improvement as it has to do with head loss and the proportionality between the discharge $(\mathrm{Q})$ and the pipe diameter (D) was highly appreciated in this paper.

Hence, the method is useful aid in designing network for maximum economy. It is believed that the findings from this work will be a great asset in planning, designing and operation of reticulated water pipeline network. The results obtained could also be applied in any part of the world since


Graph 5- The graph that shows the corrected head loss, $\mathrm{H}_{\mathrm{f}} /$ the pipe length, L .
environmental and climatic conditions do not affect the operation of the waterworks. It is hoped that if the results are applied adequately, all Nigerians should have access to potable water.

## Nomenclature

$\mathrm{Q}_{0}=$ Initial assumed flow rate $\left(\mathrm{m}^{3} / \mathrm{s}\right)$, $\Delta Q=$ Corrective discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$,
$\mathrm{D}=$ Pipe diameter (m),
$H_{L}$ or $H=$ Head losses in pipe ( $m$ of fluid),
$\mathrm{Q}=$ Flow rate through pipe, (in and out of the node) ( $\mathrm{m}^{3} / \mathrm{s}$ ),
$\mathrm{C}_{\mathrm{F}}=$ Unite conversion factor (English $=4.66, \mathrm{SI}=$
10.29),
$\mathrm{f}=$ Darcy-Weisbach friction factor,
C = Hazen-Williams coefficient,
$\mathrm{L}=$ length of pipe,
$\mathrm{n}=$ Manning roughness coefficient,
$\mathrm{i}=$ Subscript indicating location,
$j=$ Subscript indicating location,
$\Delta \mathrm{H}=$ Head difference between reservoirs in a loop

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