**Sludge granulation during anaerobic treatment of pre-hydrolysed domestic wastewater**

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**Abstract**

The aim of this study was to examine sludge granulation during the anaerobic treatment of pre-hydrolysed municipal wastewater. The pretreated wastewater had a total chemical oxygen demand (CODt) of 432 ± 20 mg/l, a soluble COD (CODs) of 259 ± 8 mg/l, volatile fatty acids (VFA) of 101 ± 9 mg/l and suspended solids (SS) of 94 ± 12 mg/l. Prior to entering the digester, the influent was supplemented with sucrose, which increased the total and soluble COD by 300 mg/l. An upflow anaerobic sludge bed (UASB) digester was operated at different hydraulic retention times (HRT) ranging from 26.7 h to 2.2 h, while the organic load rate (OLR) ranged from 0.9 to 7.3 kgCOD/m³·d. Sludge granulation was observed after day 150 of operation, at an HRT of 3.4 h, when small granules of less than 2 mm in size appeared. The granules had a weak structure and low density, with the specific methanogenic activity of the sludge being about 0.24 g CH₄-COD/gVSS·d. After granulation, the digester performance was 57% CODt removal and 76% CODs removal for steady state operation at an HRT of 3.4 h and an OLR of 5.6 kgCOD/m³·d.

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**Nomenclature**

COD: Chemical oxygen demand (t: total, s: soluble).
HRT: Hydraulic retention time
OLR: Organic load rate
SS: Suspended solids
VSS: Volatile suspended solids
VFA: Volatile fatty acids
CH₄-COD: Methane expressed as COD
UASB: Upflow anaerobic sludge bed (anaerobic digester)
HUSB: Hydrolytic upflow sludge bed (anaerobic digester)
rCH₄: Methane production rate

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**Introduction**

Anaerobic digestion has become the most commonly used method for the treatment of medium- and high-strength effluents, due to the economy of the process and the low generation of surplus sludge. Different anaerobic technologies have been applied for the treatment of less concentrated effluents, such as domestic wastewater and some industrial effluents, providing good treatment efficiencies at low hydraulic retention times (Hickey et al., 1995). One of the digester designs used for anaerobic digestion is the UASB digester, because it achieves the best results in developing and maintaining a granular sludge.

Most of the studies concentrating on clarifying the granulation process were carried out with medium to high substrate concentrations and at mesophilic (30ºC to 38ºC) or thermophilic temperatures (Hulshoff Pol, 1989; Fang et al., 1994; Quarmby and Forster, 1995). Furthermore, treatment studies at ambient temperatures were carried out by using granular sludge as an inoculum, since there is a lack of available information on granulation in digesters treating diluted wastewater at ambient temperatures (Soto et al., 1997).

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Some low-strength wastewaters like domestic and municipal effluents contain significant amounts of both fats and SS, but complex carbohydrates and VFA are among the main organic constituents (Elefsiniotis and Oldham, 1994). Several factors, such as sludge flotation and inhibition due to the effect of fats and long-chain fatty acids, or the adsorption of finely dispersed colloidal matter on the surface of the sludge, may cause the granulation process to be difficult or the granular sludge to deteriorate (Sayed, 1987; Rinzenma, 1988; Hawkes, 1995). In addition, low gradients of substrate concentration and reduced methanogenic activity at low temperatures could enhance the negative effects of these factors.

Given the contradictory results reported on the feasibility of sludge granulation treating these low-strength municipal wastewaters (Van der Last and Lettinga, 1992; Lettinga et al, 1993; Vieira et al., 1994; Ruiz et al., 1998), several granulation studies have been planned to be included as a part of a more extensive research project dealing with the anaerobic treatment of low-strength municipal wastewaters. A previous study by Soto et al (1997) reported the influence of temperature on the granulation process during the start-up of UASB digesters treating a dilute synthetic wastewater (500 mgCOD/l as sucrose) at mesophilic (30ºC) and psychrophilic (20ºC) temperatures. The results showed that the granulation process followed a similar pattern at both temperatures and complete granulation was achieved between 1 and 2 months after the start-up.

In this paper the results obtained during the start-up and granulation process in a laboratory-scale UASB digester treating a pre-hydrolysed domestic wastewater at ambient temperature (20ºC) supplemented with sucrose as a COD source (300 mg COD/l) in order to enhance granulation are reported.

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**Materials and methods**

**Anaerobic digester set-up and operation**

The UASB digester was made of Plexiglas and had an active volume of 485 ml with an internal diameter of 35 mm, and a height of 420 mm. The digester was placed in a temperature-regulated
Wastewater feed

Raw domestic wastewater was pretreated in an HUSB digester as described in Ligero et al. (2001). The characteristics of the HUSB influent and effluent are shown in Table 1. Prior to entering the UASB digester, the pre-hydrolysed domestic wastewater was supplemented with sucrose (300 mg COD/l) and sodium bicarbonate (300 mg/l), and the pH was regulated at 7.0 ± 0.1 by adding HCl.

Analytical methods

The determination of SS, VSS, COD (total), COD (soluble), fats, phosphates and sulphates was carried out according to Standard Methods (1985). Total Kjeldahl nitrogen (TKN) was determined by sample digestion with sulphuric acid and a selenium reagent, after which a Kjeldahl apparatus was used for the distillation and titration of the samples with hydrochloric acid. Ammonia was determined by using an ion-selective electrode. Biogas composition was analysed by means of gas chromatography (HP 5890 series II), using a thermal conductivity detector (Standard Methods, 1985), whereas the VFA content was analysed by gas chromatography (HP 5890 SERIE II), using a flame ionisation detector.

Results

Figure 2 shows the results of the analyses performed on the reactor effluent, while Fig. 3 presents the corresponding conversions reached, in terms of total and soluble COD removal, SS removal, and the methane generation rate in the gas phase, together with the OLR applied. These parameters are presented in Table 2 as averages for each period. Reactor operation was divided into five periods according to the HRT applied. After seeding the digester, feeding was started at a low OLR in order to allow the sludge to become acclimated to the substrate and the operational conditions. The OLR was increased step by step and the digester was operated according to Soto et al. (1997).

During Period I the digester was operated at an HRT of 26.7 h, showing total and soluble COD removal efficiencies of about 80%. The SS removal increased gradually up to 80%, reaching effluent concentrations of about 30 mg SS/l.

From day 24 to 90 the digester was operated at an HRT of 10.5 to 12.1 h (Period II). After the HRT decreased on day 24, the removal efficiencies dropped sharply, and recovered only after day 50 of operation. The effluent VFA concentration rose up to 130 mg COD/l, indicating that the methanogenic capacity of the digester was too low to accommodate the influent OLR. On other hand, the SS concentration in the effluent increased from 30 to about 100 mg/l, which was higher than the influent SS concentration. This would mean that part of the inoculated sludge was lost with the effluent.

However, the sludge balance between days 24 and 54 would indicate that the total SS going out into the effluent during this period was only about 28% of the inoculated SS. Thus, the operational conditions were maintained until the digester efficiency was recovered and stabilised, in terms of gas production and COD removal, as seen in Period IIb. From day 50 to 75 of operation, the methane production progressively increased, while the effluent VFA and SS concentrations decreased, causing the COD removal in Period IIb to be improved.

During Period III the digester was operated at an HRT of 6.1 h, reaching an average OLR of 2.8 kg COD/m²d. The digester maintained the COD removal efficiency (67%), while the methane production rate increased slightly and the effluent VFA concentration
remained low. Furthermore, the operation at this new HRT had no significant effect on the SS lost with the effluent.

On day 114 of operation the HRT was reduced to 3.5 h, and the digester was maintained in this situation until day 170 (Period IV). The OLR increased from 2.8 to 5.3 kg COD/m³·d. The digester showed an increase in effluent COD and VFA concentrations, with a decrease in the COD removal efficiency, but less than what occurred in Period IIa. The effluent SS concentration increased only momentarily (day 125 of operation), and the digester maintained the biomass retention capability.

Between days 150 to 160 of operation the digester efficiency improved, and was later maintained during Period IVb. Low effluent VFA concentrations were recorded, while the methane production rose sharply. Under these operational conditions with an HRT of 3.4 h and an OLR of 5.6 kg COD/m³·d, the digester performance was 57% CODt removal and 76% CODs removal. At this time, the presence of a mainly granular sludge in the digester was observed.

A further decrease in the HRT to 2.2 h was applied on day 171 of operation. Under these conditions there was an increase in sludge washout, leading to an increase in the VFA effluent concentration and a gradual decrease in the methane production rate. By subtracting the influent VSS concentration from the effluent VSS concentration (see average values in Table 2) and taking into account the flow through the system, the minimum amount of biomass washed out during Period V was calculated as 2.7 g VSS, which was more than twice the amount of VSS in the digester at the end of the operation (see data in next paragraph). Thus, it is clear that a biomass washout occurred during Period V.

The operation was halted on day 193 in order to determine the amount and type of sludge remaining in the digester. At this time, the digester had a sludge bed volume of 156 ml, with an average of 8.9 g SS/l and 6.8 g VSS/l. Thus, the amount of sludge in the digester was 1.4 g SS and 1.1 g VSS. This sludge maintained the granular appearance observed after day 150 of operation, presenting granules of less than 2 mm in size. During sludge handling, part of the granules were destroyed, which is an indication of their weak structure. The specific methanogenic activity of the sludge may be calculated from the rate of methane production and the sludge concentration in the digester, both measured at the end of Period V. A specific methanogenic activity of 0.24 g CH₄-COD/gVSS·d was calculated.

**Discussion and conclusions**

During the treatment of effluents composed mainly of carbohydrates in the mesophilic range of temperatures (35°C), the granulation process appears to develop slowly as the influent COD concentration decreases to below 500 mg COD/l (Ramos et al., 1994). Other researchers reported that granulation did not occur during the start-up of UASB digesters treating a synthetic wastewater composed mainly of glucose (Brito et al., 1997), or during the treatment of a glucose-supplemented domestic sewage (Gnanadiphathy and Polprasert, 1993).

However, in a previous paper it was reported that the granulation process was correctly developed during the start-up of UASB digesters treating a dilute (500 mg COD/l) sucrose-based influent (Soto et al., 1997). Complete granulation was achieved between 1 and 2 months after the start-up, at upflow superficial liquid velocities of 0.05 to 0.15 m/h and hydraulic retention times (HRT) of 6 to 3 h.
Regarding the influence of temperature, the results indicated that the granulation process followed a similar pattern at temperatures for both 30ºC and 20ºC. At 20ºC, the UASB system showed excellent stability and high treatment efficiency which were maintained even when the OLR was increased to up to 9 kg COD/m³·d, at an HRT of 1.3 h.

Anaerobic treatment of domestic and other diluted effluents should be carried out at ambient temperatures, normally in the psychrophilic range, from 10ºC to 26ºC, depending on the season and place.

There have been contradictory results on the feasibility of sludge granulation treating this low-strength municipal wastewater. The results of the majority of the studies on domestic wastewater treatment in UASB digesters indicate that these systems operate with low activity floculent sludge at ambient or psychrophilic temperatures (Lettinga et al., 1993; Ruiz et al., 1998). Some authors (Vieira et al, 1994; Van der Last and Lettinga, 1992) reported the granulation of the seed sludge during the treatment of pre-settled domestic sewage. The granular sludge was not negatively affected when the influent was changed to raw sewage (Vieira et al., 1994). However, Elmitwalli et al. (1999) reported that during the treatment of raw domestic wastewater in a digester inoculated with granular sludge, the suspended and colloidal solids contained in the wastewater caused the flotation and deterioration of the sludge. Singh et al. (1996) carried out the start-up of a pilot UASB (4 m in height) treating a dilute (300 to 500 mg COD/l) wastewater at temperatures from 25 to 30ºC. The synthetic substrate was composed of cellulose, as suspended solids (100 mg SS/l), sucrose (350 mg/l) and peptone (50 mg/l). The OLR ranged from 1 to 4 kg COD/m³·d and the HRT from 3 to 6 h. Under these conditions the sludge became granular, but with poor sedimentation characteristics, as the biomass retention capability of the digester was low at an HRT of 3 h and an upflow velocity of 1.3 m/h.

In this study, the influent to the UASB digester was a pre-hydrolysed domestic wastewater supplemented with sucrose, having an SS concentration of less than 100 mg/l. The nature and concentration of the biodegradable substrate were similar to those described in Soto et al. (1997), using only sucrose as the carbon source. Thus, the differences found in relation to the granulation process could be attributed to the effect of both suspended or colloidal solids, or to the influence of other wastewater characteristics, but not to the type of biodegradable carbon source, COD concentration or temperature.

These results indicate that granulation during the start-up of a UASB digester treating this pre-hydrolysed, sucrose-supplemented, domestic wastewater developed more slowly than during the treatment of a sucrose-based synthetic effluent. Granular sludge was not observed until 150 d after the start-up, in comparison with only 60 d in the digester treating synthetic effluent. Furthermore, the granules formed were smaller and less resistant, although their methanogenic activity was quite similar (0.25 to 0.35 g CH₄-COD/g VSS-d). The operation of the UASB digester treating the sucrose-supplemented hydrolysed effluent was stable up to an HRT of 3.4 h, while the UASB treating the synthetic effluent was able to

### TABLE 2
Operation and conversion characteristics of the UASB digester

<table>
<thead>
<tr>
<th>Period</th>
<th>I</th>
<th>IIa</th>
<th>IIb</th>
<th>III</th>
<th>IVa</th>
<th>IVb</th>
<th>V</th>
</tr>
</thead>
<tbody>
<tr>
<td>Days</td>
<td>1-23</td>
<td>23-50</td>
<td>51-99</td>
<td>99-114</td>
<td>114-158</td>
<td>158-170</td>
<td>170-193</td>
</tr>
<tr>
<td>HRT (h)</td>
<td>26.73</td>
<td>12.09</td>
<td>10.48</td>
<td>6.11</td>
<td>3.45</td>
<td>3.40</td>
<td>2.21</td>
</tr>
<tr>
<td>OLR (kgCOD/m³·d)</td>
<td>0.90</td>
<td>1.56</td>
<td>2.19</td>
<td>2.64</td>
<td>5.14</td>
<td>5.55</td>
<td>7.35</td>
</tr>
</tbody>
</table>

| Influent | | | | | | |
| pH | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 | 7.0 |
| CODt (mg/l) | 748 | 818 | 813 | 688 | 710 | 678 | 681 |
| CODs (mg/l) | 531 | 512 | 572 | 549 | 551 | 545 | 555 |
| SS (mg/l) | 125 | 121 | 124 | 75 | 95 | 78 | 75 |
| VSS (mg/l) | 105 | 105 | 109 | 68 | 86 | 71 | 66 |
| VFA (mgCOD/l) | 82 | 86 | 114 | 100 | 120 | 115 | 93 |
| %AcH | 93 | 85 | 88 | 83 | 88 | 89 | 87 |

| Effluent | | | | | | |
| pH | 7.08 | 7.13 | 7.31 | 7.31 | 7.10 | 6.88 | 6.62 |
| CODt (mg/l) | 169 | 377 | 258 | 223 | 313 | 304 | 338 |
| CODs (mg/l) | 106 | 218 | 145 | 111 | 193 | 133 | 193 |
| SS (mg/l) | 43 | 96 | 80 | 70 | 77 | 96 | 95 |
| VSS (mg/l) | 39 | 80 | 66 | 64 | 70 | 90 | 88 |
| VFA (mgCOD/l) | 8 | 98 | 42 | 15 | 71 | 8 | 75 |
| %AcH | 87 | 67 | 75 | 77 | 55 | 69 | 66 |

| Conversion | | | | | | |
| rCH₄ (l/l·d) | 0.003 | 0.006 | 0.14 | 0.18 | 0.17 | 0.54 | 0.34 |
| %CODt | 78 | 52 | 67 | 67 | 56 | 57 | 51 |
| %CODs | 81 | 55 | 75 | 79 | 65 | 76 | 65 |
| %SS | 50 | -9 | 18 | 4 | 12 | -15 | -22 |
| %VSS | 52 | -3 | 25 | -3 | 13 | -19 | -30 |
reach a minimum HRT of 1.3 h. In the first case, the key to digester instability would appear to be related to its ability to retain biomass and, thus, to the sedimentation characteristics of developed sludge. These results agree only partially with the findings of Singh et al. (1996), who reported granules that were larger and stronger treating synthetic effluents. Although the SS concentration in the influent was similar in both cases, there were differences between the type of SS, since the SS coming from the pre-hydrolysed domestic wastewater were in part colloidal solids. Moreover, pre-hydrolysed domestic wastewater contains other particulate and soluble substances that may influence granulation.

In conclusion, several factors may hinder the granulation process during the anaerobic treatment of diluted wastewaters with UASB digesters. The viability of granulation during the anaerobic treatment of municipal or domestic wastewater, even when pre-hydrolysed, is not clear and more research is needed. These future studies should focus on the factors that control the dynamic equilibrium between both granular and non-granular sludge fractions. Efficient biomass retention equipment and digester designs which contribute to this objective are also needed.

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