

# A scenario-based multiple attribute decision-making approach for site selection of a wastewater treatment plant: Bahir Dar City (Ethiopia) case study

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## ABSTRACT

The challenge of selecting an appropriate wastewater treatment plant (WWTP) site is addressed in this study, using the case study of Bahir Dar City, Ethiopia. An innovative approach is proposed for selecting the WWTP site. Fifteen practically feasible alternatives were identified based on the geographical information system (GIS) based hydraulic design and considering the overall economy of the system. The three dimensions of sustainability were considered while evaluating alternatives through incorporating economic, social, energy and environmental criteria in decision-making. The multiple attribute decision making (MADM) method was applied to prioritize the alternatives. Four scenarios representing the different perspectives of choices were used for weight elicitation. The results of the study show that the developed decision-making approach identified practically feasible alternatives. The framework and decision-making methodology developed in this study helped to facilitate the decision making by local government in a holistic view that incorporates environmental management in the city of Bahir Dar.

**Keywords:** urban planning, multiple attribute decision making, sewage outfall, sewerage system, site selection, wastewater treatment plant

## INTRODUCTION

Globally, it is well-documented that increasing population size, rapid rates of urbanization, industrialization, and consequent increasing demands on water supply result in generation of polluted effluent and solid waste. At the same time, countries are still battling to increase water and sanitation coverage in order to meet demand (WHO/UNICEF, 2017).

For example, in Ethiopia, a study by Teshome (2007) revealed that water supply and sanitation coverage is one of the lowest in the world. Additionally, the 2015 report by WHO/UNICEF (2015) indicated that there had been moderate progress on sanitation coverage between 1990 and 2015, with the proportion of the 2015 population that gained access since 1990 sitting at 27%. The particularly low sanitation coverage for Ethiopia has also been reported in earlier studies (Lüthi et al., 2010; Moe and Rheingans, 2006; WHO/UNICEF, 2006). Since sanitation and disease burden are correlated, in Amhara region (where Bahir Dar City is located, in the northwestern part of Ethiopia) with a population of about 19 million, approximately 90 000 children under 5 years of age die annually from diseases related to poor sanitation.

In Bahir Dar City, sanitation technologies like septic tanks, pit latrines, and ventilated improved pit latrines (VIP) have been declared to not be environmentally friendly. Moreover, Bahir Dar City has no centralized or decentralized wastewater collection, conveyance nor treatment technology. Hence, establishments discharge untreated wastewater directly into the nearby water bodies, with partial or no treatment. The challenge is then that improperly dumped faecal material

severely impacts the quality of ground and surface water resources (Kumie and Ali, 2005; O'Loughlin et al., 2006; Tilahun and Collick, 2011). In this context, it is essential for urban local bodies (ULBs) in Ethiopia to develop adequate sewerage and sanitation infrastructure facilities. The first step towards this would be to plan and design such facilities.

Thus, the present study focused on selection of a site for a wastewater treatment plant (WWTP) for Bahir Dar City. In this city, the available quantity of water is sufficient to meet the demand of the entire population; however, the available water resources have not been utilized efficiently. Besides, there is a long list of water-related problems, including lack of sanitation as well as pollution of surface and ground water (HEHECE, 2000). Hence, the challenges abound to increase food supplies and preserve environmental amenities because of inadequate freshwater conveyance systems and lack of wastewater treatment and reclamation systems. For Bahir Dar City, it is essential to devise an optimized sanitation system to mitigate against pollution of freshwater and enhance production of adequately recycled wastewater, which will considerably reduce ecological pressure on Lake Tana and the Blue Nile River. For these reasons, it becomes vital to choose the most suitable wastewater treatment plant sites, considering specific criteria and indicators.

Site selection has been a widely evaluated problem for many other facilities such as landfill site selection in solid waste management (Banar et al., 2007; Javaheri et al., 2006; Paul, 2012; Sumathi et al., 2008), nuclear power plant site selection (Kurt, 2014), and site selection of aquifer recharge with reclaimed water (Pedrero et al., 2011). Specifically, for on-site selection for WWTPs, there are limited studies reported in the literature. For those that are available, some have proposed a heuristic screening method for regional planning of wastewater treatment systems where the optimal location is selected based on transportation cost

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and regionalization efficiency. For example, Makropoulos et al. (2007) reported the use of a spatial decision support tool (DST) for optimal siting of wastewater treatment technologies. The tool was based on fuzzy logic and multi-criteria decision making (MCDM), which incorporated uncertainty and decision makers' attitudes towards risk in the decision making problem. Makropoulos et al. (2007) developed a DST based on MCDM for siting new WWTPs. A set of general, land-use planning, geomorphological, and hydrogeological criteria, and also a few specialized criteria, was used for the evaluation.

A novel approach is proposed in this work, integrating hydraulic model outcomes with indicators accounting for technical, economic and environmental criteria. The present approach differs from those of past studies in two major perspectives. Firstly, the sites considered for the evaluation were identified from an optimal geographical information system (GIS) based hydraulic design of a sewerage system in Bahir Dar City. Secondly, all possible practical criteria were considered for the evaluation.

## MATERIALS AND METHODS

The overall method flowchart is presented in Fig. 1. Many factors such as field feasibility, energy consumption, cost and social issues were considered while developing methods for WWTP site selection. As shown in Fig. 1, the methodology is based on multiple tools and techniques of advanced planning and management. The following manuscript sections describe the method step-by-step.

### Geographical data and demographic analysis

Bahir Dar City is located approx. 578 km north-northwest of Addis Ababa in the north-western region of Ethiopia, on the southern coast of Lake Tana. The geographical location of Bahir Dar City lies between longitude 37°20'0" E and 37°28'0" E and latitude 11°30'40" N and 11°36'35" N. The city is at the emerging point of the Blue Nile River and is surrounded by ancient monasteries, which are tourist attractions. The study area consists of 4 zones. According to the 2006–2007 population and housing census results, the population of the city was 180 770 (CSA, 2006-7). The predicted growth rates and zone wise populations are given in Tables A1 and A2 in the Appendix, respectively. The city has an approximate area of 42 000 ha of which 2 258 ha (17.2%) are covered with water bodies. Of the total land area, 3 842 ha are suitable for construction while the rest is unused land (BDIDP, 2006). The land use map, road network map, river map, digital elevation model (DEM) map, and soil map were used for this study.

Wastewater management practices in Bahir Dar City are not adequate and there is need for large infrastructure development. Most of the existing individual housing does not have access to a sewer network but rather uses pit latrines and septic tanks. Moreover, wastewater-generating institutions do not have their own wastewater treatment and management systems. Their wastewater disposal provisions are not different from those of individual housing units in the city. They also discharge wastewater into Lake Tana and the Nile River through pipes and open ditches. Such discharges pollute the environment, create offensive smells and aggravate the conditions for spreading communicable diseases (Mekonnen, 2012; Suominen et al., 2010).

Sources of sanitary wastewater	Design sanitary load/flow (MLD)	Bahir Dar City per capita water supply demand (LPCD)
Residential area	53.18	135
Commercial area	2.37	15
Small manufacturing plant*	3.98	25
Total Sanitary load/Flow	59.53	

\*Small manufacturing plants are very limited in the city and this study only considered the beverage and food processing factories (semi industrial wastes that will be treated along with municipal waste).

### Hydraulic design (identification of possible sites)

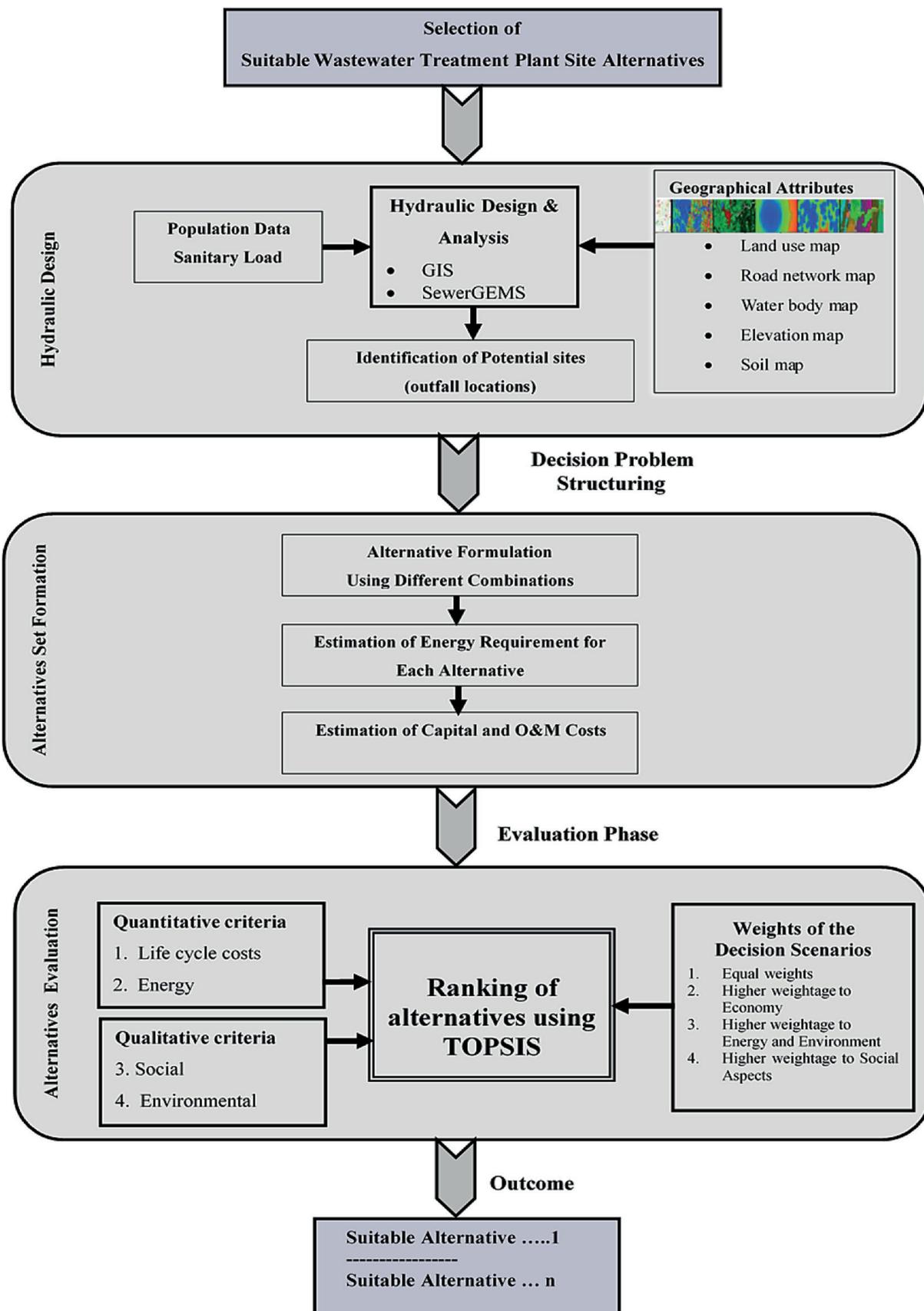
Bahir Dar City sewerage system design was carried out in order to identify possible sites for a WWTP (outfalls). The wastewater generation was estimated for residential, commercial and small-scale industrial or manufacturing plants based on population projections. The average dry-weather flow was calculated based on per capita water supply demand of Bahir Dar City. It was assumed that 80% of water supply was generated as sanitary wastewater and the peak dry-weather flow was obtained by multiplying the average dry-weather flow by the peaking factor of 2.25 for the design population. The estimated load from each service is shown in Table 1.

The hydraulic design was carried out using ArcGIS and SewerGEMS software. Projected population (Table A2, Appendix), land-use map, elevation map, road network map and current condition of the city were taken into consideration for the sewerage system design and analysis. For the purposes of this study, the locations of sewage outfalls shown in Fig. 2 were decided based on a flow accumulation map (final pouring points, i.e., lowest elevation). Based on these locations (outfalls) the sewerage network was designed and simulated in SewerGEMS. Thus, Fig. 2 shows the outcome of the hydraulic model.

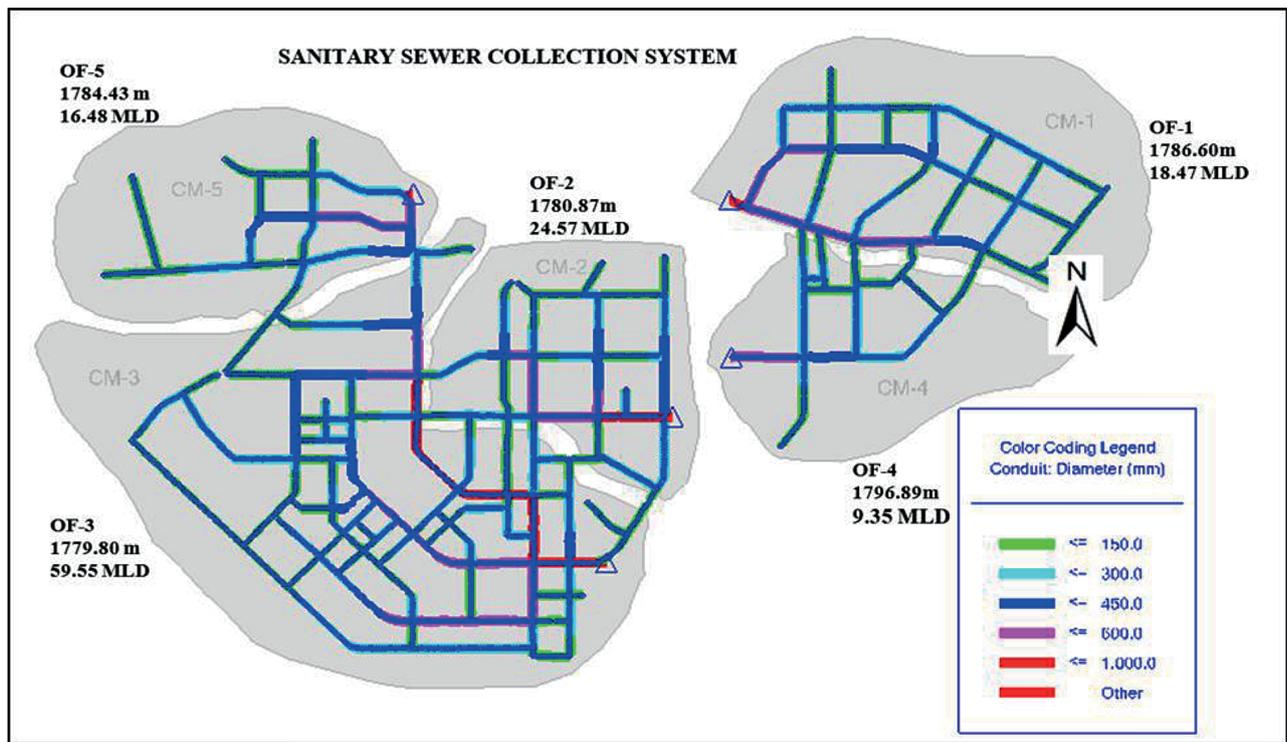
### Articulation of possible alternatives

As shown in Fig. 2, 5 possible locations for WWTPs were identified. After screening the results from hydraulic design, it was found that it was not possible to transfer the wastewater from the location of Outfall 5 due to the U shape of the lake in between OF-5 and other outfalls (simply, OF-5 is located north-west of the lake while other outfalls extend from north-east to the southern part of the city). Hence, it was decided on this outfall as one separate alternative WWTP site without interconnection with the other outfalls.

The next question addressed was whether, for the remaining 4 outfalls, the WWTP needed to be constructed at 1 location, such that wastewaters from other locations could be transferred to a common place, or whether WWTPs should be constructed at multiple locations. This led to formulation of multiple combinations of alternatives for the evaluation. Various possible combinations of alternatives were formulated by changing the



**Figure 1**  
Framework on GIS based AHP-TOPSIS approach for WWTP site selection



**Figure 2**  
Designed sanitary sewer collection system and associated drainage zones

number of WWTPs and locations. Table 2 shows the list of 15 possible alternatives and corresponding cost analyses.

Key data used for the estimation of cost shown in Table 2 are projected sanitary load/flow rate, pumping time, total pumping head and electricity cost. The inter-location transfer path from one outfall to the other is decided by the minimum cost approach. Hence, all 15 alternatives listed in Table 2 are the most practically feasible alternatives requiring minimum energy and minimum capital costs. Rigorous field information about all 4 outfall locations was obtained through field visits and data collection. Table 3 thus shows the information gathered on the 4 locations, which helped in formulation of alternatives and understanding the operation of these alternatives.

### Criteria and indicators

Various aspects need to be taken into account for selecting the site for a WWTP. Apart from technical feasibility, the three dimensions of sustainability need to be considered as well. Various sustainability criteria and indicators such as technical efficiency, minimization of environmental impacts and health risks, cost efficiency, public participation and acceptability have been proposed and applied for the site selection problem (Azar, 2000; Benedetti et al., 2010; Flores-Alsina et al., 2008; Sharifi and Retsios, 2004; Sumathi et al., 2008; Yoon and Hwang, 1995; Zhao and Yang, 2009). Based on the comprehensive literature review and considering the available data, 4 criteria were selected to rank wastewater treatment plant site alternatives. Each of the criteria has one or more indicators (attributes) shown in Table 4.

The energy consumption requirements for each alternative were estimated using design flow rates, hydraulic design results and downturn levels of the outfall locations. The energy required for local pumping, meaning wastewater transfer from outfall invert level to WWTP inlet level was estimated using dynamic hydraulic head difference available from hydraulic designs. Similarly, energy required for inter-location transfer, meaning transfer of wastewater from one outfall location to another (for example, in Alternative 6 in Table 2, wastewater needs to be transferred from OF-3 to OF-2) was estimated. Table A3 (Appendix) provides detailed information about local pumping energy and inter-location transfer energy requirements for all 15 possible alternatives.

Further, it was assumed that energy required for WWTP operation is 25% of the total energy required for pumping (Kadar and Siboni, 1998; Michel et al., 1969; Tsagarakis et al., 2003). The sum of local pumping, inter-location transfer and WWTP operation energy requirement in kWh/yr was used as an indicator representing criteria for energy consumption. Table 5 shows the summarized score for all alternatives and criteria.

Economic criteria expressed as net present value (NPV) were quantified as per the present worth method, prescribed in the Indian Standards (BIS, 1994). NPV accounts for both capital costs and operation and maintenance (O&M). The capital costs included the cost incurred for civil works, electromechanical equipment and the cost of land. It was assumed that capital costs are 25% of total operational costs required for the particular alternative (Kadar and Siboni, 1998; Michel et al., 1969; Müller, 2003; Tsagarakis et al., 2003). O&M costs included electrical energy costs required to operate the WWTP, inter-location transfer, local pumping, labour costs,

**TABLE 2**  
**Possible alternatives and corresponding cost analyses**

Alternative No.	STP alternatives	Local pumping -OF	Inter location transfer-OF	Inter location transfer (USD/yr)	Local pumping (USD/yr)	Capital cost	STP O&M cost (USD/yr -19% of total operating cost)	Miscellaneous (USD/yr -5% of total operating cost)
1	WWTP at all 4 locations	OF-1			172 141	43 035	32 707	8 607
		OF-2			229 001	57 250	43 510	11 450
		OF-3			555 114	138 778	105 472	27 756
		OF-4			87 134	21 783	16 555	4 357
	<b>NPV</b>				<b>8 183 446</b>	<b>2 045 861</b>	<b>1 554 855</b>	<b>409 172</b>
2	WWTP at OF-1	OF-1	OF-2	305 118	172 141	119 315	90 679	23 863
		OF-1	OF-3	884 558	172 141	264 175	200 773	52 835
		OF-1	OF-4	58 117	172 141	57 565	43 749	11 513
	<b>NPV</b>			<b>9 786 616</b>	<b>4 050 380</b>	<b>3 459 249</b>	<b>2 629 029</b>	<b>691 850</b>
3	WWTP at OF-2	OF-2	OF-1	112 294	229 001	85 324	64 846	17 065
		OF-2	OF-3	325 642	229 001	138 661	105 382	27 732
		OF-2	OF-4	51 115	229 001	138 661	105 382	27 732
	<b>NPV</b>			<b>3 835 699</b>	<b>5 388 253</b>	<b>2 844 278</b>	<b>2 161 651</b>	<b>568 856</b>
4	WWTP at OF-3	OF-3	OF-1	130 836	555 114	151 557	115 183	30 311
		OF-3	OF-2	156 820	555 114	177 983	135 267	35 597
		OF-3	OF-4	65 530	555 114	155 161	117 922	31 032
	<b>NPV</b>			<b>2 770 086</b>	<b>13 061 500</b>	<b>3 801 581</b>	<b>2 889 202</b>	<b>760 316</b>
5	WWTP at OF-4	OF-4	OF-1	281 674	87 134	38 166	29 006	7 633
		OF-4	OF-2	503 061	87 134	147 549	112 137	29 510
		OF-4	OF-3	1 369 192	87 134	364 081	276 702	72 816
	<b>NPV</b>			<b>1 689 3546</b>	<b>2 050 204</b>	<b>4 312 126</b>	<b>3 277 216</b>	<b>862 425</b>
6	WWTP at OF-1 & OF-2	OF-1	OF-3					
		OF-2		432 750	229 001	165 438	125 733	33 088
		OF-1	OF-4					
		OF-2		51 115		12 779	9 712	2 556
	<b>NPV</b>			<b>3 795 014</b>	<b>1 796 084</b>	<b>1 397 775</b>	<b>1 062 309</b>	<b>279 555</b>
7	WWTP at OF-1 & OF-3	OF-1	OF-2					
		OF-3		156 820	555 114	177 983	135 267	35 597
		OF-1	OF-4					
		OF-3		58 117	172 141	57 565	43 749	11 513
	<b>NPV</b>			<b>1 685 787</b>	<b>5 703 960</b>	<b>1 847 437</b>	<b>1 404 052</b>	<b>369 487</b>
8	WWTP at OF-1 & OF-4	OF-1	OF-2					
		OF-4						
		OF-1	OF-3					
		OF-4		884 558		221 140	168 066	44 228
	<b>NPV</b>			<b>9 330 792</b>	<b>3 483 067</b>	<b>3 203 465</b>	<b>2 434 633</b>	<b>640 693</b>
9	WWTP at OF-2 & OF-3	OF-2	OF1					
		OF-3		104 754	555 114	164 967	125 375	32 993
		OF-2	OF-4					
		OF-3		51 115	229 001	70 029	53 222	14 006
	<b>NPV</b>			<b>1 222 499</b>	<b>6 149 918</b>	<b>1 843 104</b>	<b>1 400 759</b>	<b>368 621</b>
10	WWTP at OF-2 & OF-4	OF-2	OF-1					
		OF-4						
		OF-2	OF-3					
		OF-4		432 750		108 187	82 222	21 637
	<b>NPV</b>			<b>4 274 857</b>	<b>1 796 084</b>	<b>1 517 735</b>	<b>1 153 479</b>	<b>303 547</b>
11	WWTP at OF-3 & OF-4	OF-3	OF-1					
		OF-4		104 754	555 114	137 211	104 281	27 442
		OF-3	OF-2					
		OF-4		156 820		39 205	29 796	7 841
	<b>NPV</b>			<b>2 051 563</b>	<b>4 353 833</b>	<b>1 383 657</b>	<b>1 051 580</b>	<b>276 731</b>
12	WWTP at OF-1, OF-2 & OF-3	OF-1	OF-4					
		OF-2		51 115	229 001	70 029	53 222	14 006
		OF-3						
	<b>NPV</b>			<b>400 899</b>	<b>1 796 084</b>	<b>549 246</b>	<b>417 427</b>	<b>109 849</b>
13	WWTP at OF-1, OF-2 & OF-4	OF-1	OF-3					
		OF-2		432 750	229 001	165 438	125 733	33 088
		OF-4						
	<b>NPV</b>			<b>3 394 115</b>	<b>1 796 084</b>	<b>1 297 550</b>	<b>986 138</b>	<b>259 510</b>
14	WWTP at OF-1, OF-3 & OF-4	OF-1	OF-2					
		OF-3		156 820	555 114	177 983	135 267	35 597
		OF-4						
	<b>NPV</b>			<b>1 229 963</b>	<b>4 353 833</b>	<b>1 395 949</b>	<b>1 060 921</b>	<b>279 190</b>
15	WWTP at OF-2, OF-3 & OF-4	OF-2						
		OF-3						
		OF-4	104 754	555 114	164 967	125 375	32 993	
	<b>NPV</b>			<b>821 600</b>	<b>4 353 833</b>	<b>1 293 858</b>	<b>983 332</b>	<b>258 772</b>

Outfall	Area (ha)	Surrounding plots
OF-1	7.00	Agricultural plots – horticulture
	3.25	Administration
	16.04	Public service
	14.68	Formal green
	0.75	Recreation – open space
	10.20	Lake Tana
	0.48	Commercial centr
OF-2	60.18	Open ditch/flood-prone area
	26.29	Forest & informal green
	19.17	Conservation area
	14.52	Commercial activities
	17 600 m length	Blue Nile River
OF-3	13.42	Conservation area
	36.19	Open space
	102.3	Agricultural and informal green
	27.95	Manufacturing and storage
	17.04	Residential area (peri-urban area far from outfall site, about 1.5 km)
	17 600 m length	Blue Nile River
OF-4	31.99	Recreation – formal green
	17 600 m length	Blue Nile River
	6.87	Agricultural – horticulture
	19.30	Forest and informal green
	2.25	Recreation – resort centres

spare parts and maintenance costs. It was assumed that O&M costs are 19% and miscellaneous costs are 5% of the total operational costs (Kadar and Siboni, 1998; Michel et al., 1969; Müller, 2003; Tsagarakis et al., 2003).

Social criteria are often neglected from decision making for site selection. However, this is one of the important criteria as it considers compatibility of the facility in terms of the distance from human settlement, public benefit and mitigation of impacts to the host community, by evaluating the present and anticipated future use of the surrounding area where the new site is proposed (Lober, 1995). As shown in Table 4, in this study 3 indicators were used for incorporating social issues in the WWTP site selection problem.

Land acquisition is one of the major challenges in any developmental project. Hence, it is important to consider this indicator in decision making. Land use permitting or zoning shall be compatible with the host neighbourhood and sufficiently flexible to enable opportunities for vehicle movement and infrastructure development. Furthermore, this indicator has focused on change from existing use and minimal displacement of housing and businesses. Each community has a different culture, attitudes, and habits for the reuse of waste and sanitation and hence acceptability is a crucial socio-economic factor for the selection of a WWTP site. In this study the acceptability indicator represents these social acceptance issues along with aesthetics of the surrounding area and distance from the residential zones and public buildings (schools, hospitals).

The third indicator in social criteria incorporates the issues of local development and creation of public participation. This is one of the important aspects to make any developmental project successful. If the local development goes hand-in-hand with infrastructure development projects, then the projects will become successful on a social front, with enhanced sustainability.

The local environmental aspect is an important consideration in any site selection problem. Environmental criteria cover surface water and groundwater quality, land cover and ecological character, site management and public health (Paul, 2012; SAHC, 1998; Sumathi et al., 2008). In this

No.	Criteria	Indicator	Weights			
			Equal weights	Higher weighting to economy	Higher weighting to energy and environment	Higher weighting to social aspects
1	Economic (life cycle costs)	Net present value (USD) <sup>1</sup>	0.333	0.600	0.200	0.200
2	Energy consumption <sup>2</sup>	Energy consumption (kWh/yr)	0.167	0.200	0.300	0.100
3	Social	Land acquisition	0.111	0.033	0.067	0.200
		Acceptability	0.111	0.033	0.067	0.200
		Local development & public participation	0.111	0.033	0.067	0.200
4	Environmental	Water quality deterioration	0.167	0.100	0.300	0.100

Note 1: Net present value comprises the capital costs and O&M costs (inter-location pumping, local pumping and miscellaneous costs)

Note 2: Energy consumption includes inter-location pumping, local pumping and WWTP operating energy

Alternatives	Criteria					
	Energy (kWh/year)	NPV (USD)	Land acquisition	Acceptability	Local development & public participation	Water quality deterioration
1	21 644 487	12 193 334	900	500	400	800
2	36 509 272	20 617 124	750	350	300	700
3	26 576 519	14 798 736	600	500	450	100
4	42 632 663	23 282 685	300	650	600	100
5	50 178 410	27 395 518	650	400	300	200
6	14 806 028	8 330 737	675	425	375	400
7	19 549 064	11 010 723	525	500	450	400
8	28 231 192	19 092 649	750	350	300	700
9	19 502 202	10 984 901	450	575	525	100
10	20 719 885	9 045 702	600	500	450	100
11	16 938 692	9 117 365	300	650	600	100
12	57 80 731	3 273 505	600	500	450	100
13	13 723 325	7 733 398	600	500	450	100
14	14 794 505	8 319 857	300	650	600	100
15	13 721 471	7 711 396	300	650	600	100
<b>Type of criteria</b>	Cost	Cost	Cost	Benefit	Benefit	Cost

study, due to data availability considerations, deterioration of local water resources quality is considered as an indicator. The selected wastewater treatment plant site alternatives might discharge the treated or untreated wastewater, i.e., overflow sewage into the neighbouring water body. This particular attribute attempts to account for the proximity to groundwater and surface water, reclaimed water to river outfall, and potential raw sewage discharge into the water.

As is evident from the list of indicators in Table 5, the indicators NPV and energy consumption are quantitative. The indicators accounting for social and environmental issues are qualitative. Hence, a different approach is needed to quantify the qualitative indicators. Land acquisition, acceptability, local development and public participation, and water quality deterioration were quantified using a cardinal scale (1–1 000) based on authors' field visits, expertise sought, discussion with municipal authorities and native knowledge about Bahir Dar City (Table 3).

### Weight elicitation

Weighing is an important part of MADM, which affects the results. Weight elicitation can be carried out using various approaches. Use of direct rating using cardinal scales or the AHP method is common (Saaty, 1980; Saaty, 1990) for weight elicitation. Direct rating was adopted in this study for weight elicitation. Also, it is important to structure the decision problem correctly (formulation of scenarios) to obtain stable ranking results using MADM (Kalbar et al., 2012). Hence, in this study, as shown in Table 6, we used 4 sets of weights representing 4 scenarios. The 4 scenarios include equal weights to 3 criteria (economy, energy and environment and social issues), higher weight to economy, higher weight to energy and environment and higher weight to social issues. Considering

these 4 scenarios, weight elicitation using a direct rating method was carried out for all the indicators.

### Ranking of alternatives with TOPSIS

Many MADM methods are available for ranking the alternatives (Yoon and Hwang, 1995). In the context of environmental decision making TOPSIS has been used as the most suitable method for evaluation of alternatives (Kalbar et al., 2012). The ability of TOPSIS to consider positive and negative aspects of the alternative simultaneously makes the method more suitable for environmental problems. The intuitive nature of human thinking to strike the balance between cost and benefit types of indicators is mimicked in TOPSIS. Hence, TOPSIS has been chosen for the ranking in this study. The detailed method and theory behind TOPSIS is described in Kalbar et al. (2012).

## RESULTS AND DISCUSSION

Site selection problem for WWTP are addressed in this study. This complex decision-making problem is structured in 3 layers. First, the hydraulically feasible WWTP locations (outfalls) were identified using GIS-based hydraulic design. Then, all the practically possible combinations were worked out in order to formulate the set of 15 possible alternatives. Finally, these 15 alternatives were evaluated using the MADM method in order to identify the best alternative.

Results of the ranking using the TOPSIS method are presented in Table 6. Thus, for the 3 scenarios, Alternative 12 (WWTP at OF-1, OF-2 and OF-3) was identified as the best alternative. Further, it is interesting to note that Alternative 15 is ranked as 2<sup>nd</sup> in the first 3 scenarios. For the scenario

**TABLE 6**  
**Results of the ranking using TOPSIS methodology**

Rank	Equal weights		Higher weighting to economy		Higher weighting to energy and environment		Higher weighting to social aspects	
	Alternatives	Score	Alternatives	Score	Alternatives	Score	Alternatives	Score
1	A12	0.9044	A12	0.9797	A12	0.9493	A15	0.8672
2	A15	0.8438	A15	0.8202	A15	0.8730	A14	0.8500
3	A14	0.8231	A13	0.8183	A13	0.8636	A11	0.8253
4	A13	0.8152	A14	0.7957	A14	0.8566	A12	0.7670
5	A11	0.7934	A6	0.7805	A11	0.8278	A9	0.7334
6	A10	0.7579	A11	0.7619	A9	0.7857	A13	0.7181
7	A9	0.7283	A10	0.7538	A10	0.7831	A10	0.6850
8	A6	0.7131	A9	0.6886	A3	0.6903	A7	0.6323
9	A7	0.6541	A7	0.6765	A6	0.6771	A6	0.6055
10	A3	0.5998	A1	0.6060	A7	0.6314	A3	0.5761
11	A1	0.4929	A3	0.5367	A4	0.5326	A4	0.5171
12	A4	0.4118	A8	0.3553	A5	0.4375	A1	0.4310
13	A8	0.3312	A2	0.2801	A1	0.3915	A5	0.3012
14	A5	0.3003	A4	0.2315	A8	0.3283	A8	0.2959
15	A2	0.2586	A5	0.1326	A2	0.2366	A2	0.2394

in which a higher weighting was given to the social criteria, Alternative 15 was identified as the best one.

The preference for Alternative 12 can be justified in the context of site-specific information given in Table 3. The site-specific information reveals that the relative open area available at each of the outfalls varies drastically. The outfalls (OF-1 to OF-4) have relative open areas of 12%, 28%, 46% and 14%, respectively. Hence, the results of ranking match with the advantages and disadvantages posed by the locations of each site. Selection of Alternative 12 implies that all the wastewater collected at OF-4 (9.35 MLD) shall be transferred to OF-2 and a bigger WWTP constructed of about 33.92 MLD capacity at OF-2, which is feasible looking at the 28% of relative open area at OF-2. Similarly, a bigger WWTP can be constructed at OF-3, if Alternative 15 is implemented. In any case, the evaluation of alternatives shows that a WWTP at OF-4 should not be implemented, the reasons for this being the small surrounding area near OF-4 and low wastewater flow. Also, it seems logical that as the wastewater quantity collected at OF-4 is small it can be easily transferred to a nearer outfall, which is evident from the ranking results.

Additionally, Alternative 2 is ranked 15<sup>th</sup> for 3 scenarios, showing that constructing a single large WTTTP at OF-1 is not at all desirable. This can be due to the sensitive local social and environmental conditions. Alternative 5 ranked lowest of the scenarios where higher weighting to economy was given. This justifies that as the wastewater flows at other outfall locations are high, transferring wastewater from these outfalls to OF-4 will not be economical. Overall it was found that constructing a large WWTP at OF-1 and OF-4 is not feasible from social, economic and environmental perspectives.

Hence, the choice of Alternative 12 (constructing WWTPs at OF-1, OF-2 and OF-3 by transferring wastewater flow from OF-4 to OF-2) is the most appropriate strategy which matches the local conditions. In order to examine the

robustness of the final results regarding the selection of a wastewater treatment plant site, sensitivity analysis was elicited for each of the criteria and indicators with respect to alternatives. It is indispensable to analyse how each criterion affects the ranking of alternatives and to determine the weight range within which the ranking will not change. It can be noted from Table 7 that Alternative 5 (with the threshold value of -10.02%) has the minimum global criticality degree among the alternatives in the equal weight scenario for a particular economic indicator (inter-location transfer cost).

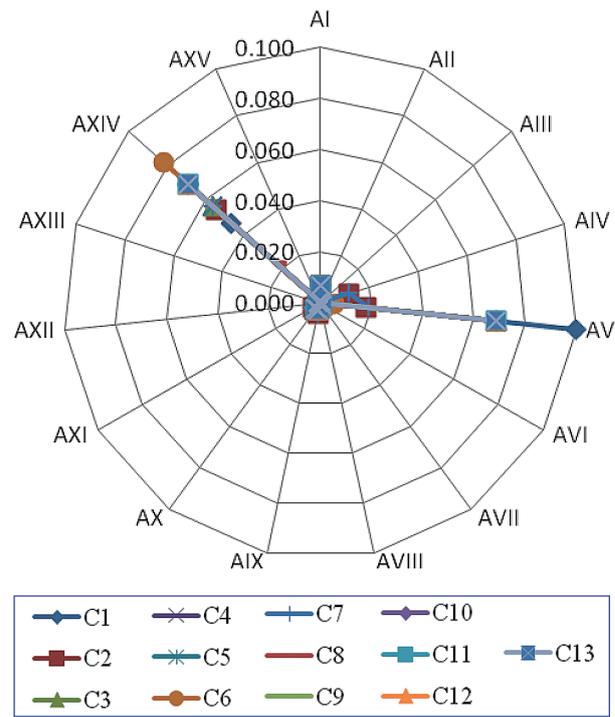
This implies that Alternative 5 is the most sensitive alternative in the equal weight scenario, and a relative increase of more than 10.02% in this indicator will affect the ranking and will lead to swapping the rank of Alternative 5 with Alternative 14. Additionally, the sensitivity coefficient for Alternative 5 is the highest, which indicates that Alternative 5 is the most sensitive alternative to the respective criterion as is clearly shown in Fig. 3. The figure also shows that the rank of Alternative 12 is not sensitive to any of the criteria and thus its rank will not change.

Although the results from the present study are interesting and reflect correctly on the field situation, there are some limitations of the study that need to be taken into account when using the results in further applications. Some gross assumptions were made while making cost estimates for capital costs and O&M costs.

Additionally, the energy consumption requirement for operation of the plant was assumed based on secondary literature. However, this does not affect the results, as it can be seen that the capital cost investments or O&M costs will follow the ratio of wastewater quantities to be treated; hence it is envisaged that the results of this preliminary analysis remain unchanged during detailed evaluation.

**TABLE 7**  
Sensitivity analysis for different indicator scores in equal weight scenario

Alternative	Indicators												
	NPV			Energy			Social			Environmental			
	Inter location transfer (USD)	Local pumping (USD)	Capital cost (USD)	O & M cost (USD)	Miscellaneous (USD)	Inter location transfer (kWh/yr)	Local pumping (kWh/yr)	Operating energy (kWh/yr)	Land acquisition	Acceptability	Affordability	Water quality deterioration	
C1	C2	C3	C4	C5	C6	C7	C8	C9	C10	C11	C12		
1	NF	-143.65	NC	NC	NC	NF	-139.2	NC	-148.14	-148.1	-148.14	-148.1	
2	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
3	NF	NF	NF	NF	NF	NF	NF	NC	NF	NF	NF	NF	
4	NC	86.96	NC	NC	NC	NC	83.92	NC	NC	NC	NC	NC	
5	-10.02	-56.24	-14.59	-14.59	-14.59	-188.32	-53.39	NC	-14.51	-14.51	-14.51	-14.51	
6	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
7	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
8	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
9	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
10	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
11	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
12	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
13	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	
14	21.49	18.35	17.78	17.78	17.78	12.20	17.74	43.06	14.42	14.42	14.42	14.42	
15	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	



**Figure 3**  
Radar plot showing the most sensitive alternatives for the particular indicators

**CONCLUSIONS**

The Bahir Dar City presently lacks water and sanitation infrastructure facilities and is in the phase of planning and development of these facilities. In this context it is highly relevant to take a systemic approach to decision making while planning and designing these facilities. The present study addresses the site selection problem for WTP at Bahir Dar City.

A three-tiered approach was adopted to solve the complex decision problem of site selection for WTPs. A GIS-based hydraulic design of the underground sewer system was carried out to identify feasible outfall locations. From these feasible outfall locations, a set of 15 possible alternatives was formulated. These 15 alternatives were evaluated in detail to quantify the economic, environmental and social performance.

The quantified indicators were used to rank these alternatives under various scenarios using MADM methodology (TOPSIS). The results from ranking showed that Alternative 12 (constructing WTPs at OF-1, OF-2 and OF-3 by transferring wastewater flow from OF-4 to OF-2) is most feasible from all three dimensions of sustainability. The results of ranking also helped to understand that, overall, constructing large WTP at OF-1 and OF-4 is not feasible from social, economic and environmental perspectives.

The developed decision-making approach is robust and provides a decision support system for municipal authorities of Bahir Dar City. The framework, in future, can be extended further to include choice of wastewater treatment technology in the context of overall planning for the city.

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**TABLE A1**  
**Annual percentage population growth rate (CSA, 1994)**

Year	1995-2000	2000-2005	2005-2010	2010-2015	2015-2020	2020-2025	2025-2030	2030-2035	2035-2040
Growth rate (%)	4.8	4.1	4.2	4	3.9	3.8	3.6	3.6	3.6

**TABLE A2**  
**Projected population for each administrative zone**

Year	2007	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026
Growth rate	0.042	0.04	0.04	0.039	0.039	0.039	0.039	0.039	0.038	0.038	0.038	0.038	0.038	0.036
Fasilo	50 406	66 330	68 983	71 674	74 469	77 373	80 391	83 526	86 700	89 995	93 415	96 964	100 649	104 272
Gish Abay	35 171	46 282	48 133	50 010	51 961	53 987	56 093	58 280	60 495	62 794	65 180	67 657	70 228	72 756
Sefene selam	35 454	46 654	48 521	50 413	52 379	54 422	56 544	58 749	60 982	63 299	65 705	68 201	70 793	73 342
Belay zeleke	59 739	78 612	81 756	84 945	88 258	91 700	95 276	98 992	102 753	106 658	110 711	114 918	119 285	123 579
Total projected Population	180 770	237 878	247 393	257 042	267 066	277 482	288 304	299 548	310 931	32 274	33 500	347 741	360 955	37 349
Year	2027	2028	2029	2030	2031	2032	2033	2034	2035	2036	2037	2038	2039	
Growth rate	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	0.036	
Fasilo	108 026	111 915	115 944	120 118	124 442	128 922	133 563	138 372	143 353	148 514	153 860	159 399	165 138	
Gish Abay	75 375	78 089	80 900	83 812	86 830	89 956	93 194	96 549	100 025	103 626	107 356	111 221	115 225	
Sefene Selam	75 982	78 717	81 551	84 487	87 528	90 679	93 944	97 326	100 830	104 459	108 220	112 116	116 152	
Belay Zeleke	128 028	132 637	137 412	142 359	147 484	152 793	158 294	163 992	169 896	176 012	182 349	188 913	195 714	
Total PP	387 411	401 358	415 807	430 776	446 284	462 350	478 995	496 239	514 103	532 611	551 785	571 649	592 229	

TABLE A3 Energy consumption for each alternatives and corresponding indicators					
Alternatives	Local pumping – OF	Inter-location transfer – OF	Inter-location transfer (kWh/yr)	Local pumping (kWh/yr)	STP energy operating (kWh/yr – 25% of total operating energy)
1	OF-1			2 818 816.88	704 704.22
	OF-2			3 758 422.50	939 605.63
	OF-3			9 261 826.88	2 315 456.72
	OF-4			1 476 523.13	369 130.78
<b>Total energy (kWh/yr)</b>			<b>0.00</b>	<b>17 315 589.38</b>	<b>4 328 897.34</b>
2 (OF-1)	OF-1	OF-2	5 007 675.56	2 818 816.88	1 956 623.11
	OF-1	OF-3	14 758 461.51	2 818 816.88	4 394 319.60
	OF-1	OF-4	984 829.85	2 818 816.88	950 911.68
<b>Total energy (kWh/yr)</b>			<b>20 750 966.91</b>	<b>8 456 450.63</b>	<b>7 301 854.38</b>
3 (OF-2)	OF-2	OF-1	1 838 825.62	3 758 422.50	2 744 664.91
	OF-2	OF-3	7 220 237.15	3 758 422.50	1 156 146.25
	OF-2	OF-4	866 162.49	3 758 422.50	1 475 214.65
<b>Total energy (kWh/yr)</b>			<b>9 925 225.26</b>	<b>11 275 267.50</b>	<b>5 376 025.81</b>
4 (OF-3)	OF-3	OF-1	2 142 436.09	9 261 826.88	2 958 901.05
	OF-3	OF-2	2 573 777.32	9 261 826.88	2 593 066.77
	OF-3	OF-4	1 110 440.20	9 261 826.88	3 468 561.26
<b>Total energy (kWh/yr)</b>			<b>5 826 653.61</b>	<b>27 785 480.63</b>	<b>9 020 529.08</b>
5 (OF-4)	OF-4	OF-1	4 612 418.17	1 476 523.13	6 080 221.56
	OF-4	OF-2	8 256 377.42	1 476 523.13	4 058 746.16
	OF-4	OF-3	22 844 363.13	1 476 523.13	2 174 190.07
<b>Total energy (kWh/yr)</b>			<b>35 713 158.72</b>	<b>4 429 569.38</b>	<b>12 313 157.79</b>
6 (OF-1 & OF-2)	OF-1	OF-3		2 818 816.88	921 244.84
	OF-2		7 220 237.15	3 758 422.50	2 191 524.51
	OF-1	OF-4		2 818 816.88	1 348 148.55
	OF-2		866 162.49	3 758 422.50	1 185 813.09
<b>Total energy (kWh/yr)</b>			<b>8 086 399.64</b>	<b>13 154 478.75</b>	<b>5 646 730.99</b>
7 (OF-1 & OF-3)	OF-1	OF-2		2 818 816.88	1 956 623.11
	OF-3		2 573 777.32	9 261 826.88	4 379 551.07
	OF-1	OF-4	984 829.85	2 818 816.88	4 394 319.60
	OF-3			9 261 826.88	8 026 547.50
<b>Total energy (kWh/yr)</b>			<b>3 558 607.17</b>	<b>24 161 287.50</b>	<b>18 757 041.28</b>
8 (OF-1 & OF-4)	OF-1	OF-2	5 007 675.56	2 818 816.88	1 133 541.66
	OF-4			1 476 523.13	585 671.40
	OF-1	OF-3	14 758 461.51	2 818 816.88	982 314.27
	OF-4			1 476 523.13	828 837.19
<b>Total energy (kWh/yr)</b>			<b>19 766 137.07</b>	<b>8 590 680.00</b>	<b>3 530 364.52</b>
9 (OF-2 & OF-3)	OF-2	OF-1		3 758 422.50	2 744 664.91
	OF-3		1 715 349.78	9 261 826.88	8 026 547.50
	OF-2	OF-4	866 162.49	3 758 422.50	1 368 443.07
	OF-3			9 261 826.88	3 468 561.26
<b>Total energy (kWh/yr)</b>			<b>2 581 512.27</b>	<b>26 040 498.75</b>	<b>15 608 216.74</b>

TABLE A3 (continued)					
Alternatives	Local pumping – OF	Inter-location transfer – OF	Inter-location transfer (kWh/yr)	Local pumping (kWh/yr)	STP energy operating (kWh/yr – 25% of total operating energy)
10 (OF-2 & OF-4)	OF-2	OF-1	1 838 825.62	3 758 422.50	3 003 699.98
	OF-4			1 476 523.13	615 338.24
	OF-2	OF-3	7 220 237.15	3 758 422.50	1 156 146.25
	OF-4			1 476 523.13	646 740.83
<b>Total energy (kWh/yr)</b>			<b>9 059 062.77</b>	<b>10 469 891.25</b>	<b>5 421 925.30</b>
11 (OF-3 & OF-4)	OF-3	OF-1	1 715 349.78	9 261 826.88	4 120 516.01
	OF-4			1 476 523.13	6 080 221.56
	OF-3	OF-2	2 573 777.32	9 261 826.88	3 567 375.61
	OF-4			1 476 523.13	1 012 575.11
<b>Total energy (kWh/yr)</b>			<b>4 289 127.10</b>	<b>21 476 700.00</b>	<b>14 780 688.29</b>
12 (OF-1,OF-2 & OF-3)	OF-1	OF-4		2 818 816.88	1 164 410.62
	OF-2		866 162.49	3 758 422.50	1 368 443.07
	OF-3			9 261 826.88	3 468 561.26
<b>Total energy (kWh/yr)</b>				<b>15 839 066.25</b>	<b>6 001 414.95</b>
13 (OF-1,OF-2 & OF-4)	OF-1	OF-3		2 818 816.88	10 838 419.28
	OF-2		7 220 237.15	3 758 422.50	20 308 395.50
	OF-4			1 476 523.13	37 148 229.74
<b>Total energy (kWh/yr)</b>				<b>8 053 762.50</b>	<b>68 295 044.52</b>
14 (OF-1,OF-3 & OF-4)	OF-1	OF-2		2 818 816.88	125 751 669.76
	OF-3		2 573 777.32	9 261 826.88	231 194 944.01
	OF-4			1 476 523.13	425 241 658.29
<b>Total energy (kWh/yr)</b>				<b>13 557 166.88</b>	<b>782 188 272.05</b>
15 (OF-2, OF-3 & OF-4)	OF-2	OF-1		3 758 422.50	1 438 624 874.35
	OF-3		1 715 349.78	9 261 826.88	2 646 054 804.69
	OF-4			1 476 523.13	4 866 867 951.09
<b>Total energy (kWh/yr)</b>				<b>14 496 772.50</b>	<b>8 951 547 630.13</b>